



March 16, 2022

SSI Project Number: 0246-002

County Administration
262075 Rocky View Point
Rocky View County, AB, T4A 0X2

Dear Sir/ Madam:

**Re: Cochrane Hamlet Conceptual Scheme (CS) Stormwater Management Plan
NW ¼ 22-26-4-W5**

This conceptual Stormwater Management Plan is in support of the Cochrane Hamlet CS located at the southeast corner of Cochrane Lake Road and Range Road 43 in NW ¼ 22-26-4-W5.

Introduction

The proposed development is located west of Highway 22, along Cochrane Lake Road just south of Cochrane Lake, as shown on **Figure 1**. The quarter section contains nine residential lots ranging in size from 4 to 28 acres. Several of these lots will be consolidated to form the project boundary covering a total area of 100 acres (40 hectares). An additional area of 20 acres (8 hectares) also discharges north towards the 100 acres and is included in the study area boundary, as shown on **Figure 2**.

The topography shows there is a general slope from southeast to north west. From here runoff flows north to Cochrane Lake, a terminal water body.

The housing density for the Hamlet is proposed to be 4 to 6 units per acre (upa). As such some innovative stormwater management techniques are proposed. There are three main stormwater proposals for the site:

- Using infrequent ponding zones as Municipal Reserve₁
- Implementing the County's alternative road sections, where possible₁
- Flow balancing with other, less dense, developments.

Design Criteria

The project lies within the Cochrane Lake catchment area boundary and, as such, follows the allowable discharge rates and volumes stated in the Cochrane Lake Sub-catchment Master Drainage Plan by SSI in 2016. These are:

- 1:100 year maximum discharge rate of 1.39 L/s/ha;
- Average Annual Runoff of 30 – 40 mm
- Removal of 85% of 50 micron Total Suspended Solids.

Figure 2 shows the study area boundary which includes drainage areas that flow through the property. The boundary of the Cochrane Lake Sub-catchment Master Drainage Plan is also shown as it runs through the development area.

In order to determine the proposed stormwater runoff from the site, it is necessary to review the native soil types. From the Geotechnical Assessment for the Proposed Subdivision Cochrane Lake Hamlet by Watt Consulting in January 2022, the site is underlain by silty clay. This means that infiltration rates are relatively low. Clay pond liners have a design infiltration rate of 1×10^{-8} m/s, in accordance with provincial and some municipal guidelines. As the native materials contain very little sand or gravel near the surface, the above rate is assumed in the design. The soil type is Type C with a Curve Number of 72 in accordance with the USDA Soil Conservation Service 1968. A more accurate assessment of the subsurface material will be provided at the next design stage.

To reduce the runoff values to those stated above it is recommended that Low-Impact Development Measures are provided. A stormwater management facility is proposed in the northwest corner of the site to reduce the peak discharge rates.

To reduce runoff volumes:

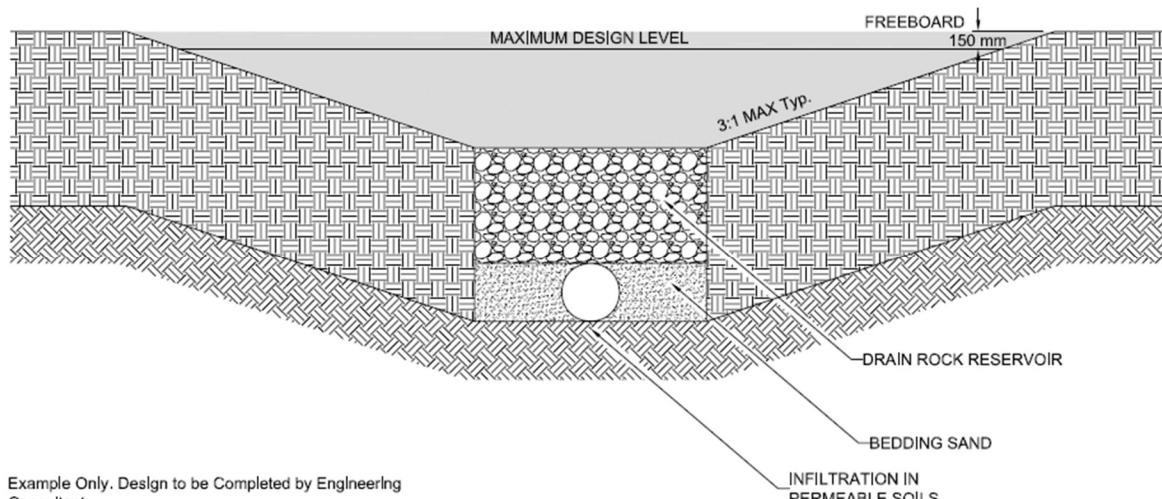
- Roadside bioswales are proposed across the development in areas of low-density housing to enhance infiltration;
- Topsoil areas will be 300 mm thick;
- All roof leaders should be directed to grassed areas.

The use of an irrigation system, re-using water from the storm pond, is an option, although the initial construction costs (by the developer) and long-term maintenance costs (by the County) have not yet been quantified.

Bioswales, or bioretention areas, are Low-Impact Development (LID) Measures which allow stormwater runoff to be infiltrated rather than to discharge offsite. They not only protect downstream watercourses from excessive runoff, but also filter the water to protect fish habitat. They are typically placed at the side of a road to collect runoff from an overland swale or catchbasin. They consist of a



sand/ gravel or other well-draining subsoil, below a deep, well-draining growing medium. Above this there is a ponding area of variable depth and a high-level discharge pipe or spillway. The figure opposite is an example of a bioretention zone. Trees, shrubs or other vegetation can be planted in the bioretention area as required. Below is a typical cross-section of a roadside bioswale.



In addition to the bioretention area, an additional LID measure is proposed. This is called absorbent landscaping. One approach to absorbent landscaping is simply adding topsoil to the proposed lots and roadway ditches. Construction of the roads and houses typically results in excessive topsoil. This can be stockpiled and used by homeowners during development of their lots. Further, from the Geotechnical Evaluation the topsoil across the site is approximately 300 mm. Excess topsoil, resulting from grading can be spread across the site. A plan of the borehole locations and logs is provided in the Geotechnical Assessment for the Proposed Subdivision Cochrane Lake Hamlet by Watt Consulting in January 2022.

Analysis

The Water Balance Spreadsheet for the City of Calgary (WBSCC) is used to simulate continuous rainfall over a 51 year period, and SWHYMO is used to simulate the 1:5 year and 1:100 year design storms. It is recommended that the 24-hour storm duration is used here.

The site layout is shown on **Figure 2** and are detailed in **Table 1** below.

Table 1 – Catchment Properties

Surface	Area (m ²)	% Imp	% Imp to grass
Residential	25.50	40	100
Roads	16.41	65	50
MR	4.80	5	100
Pond/ PUL	1.01	100	0
TOTAL	47.72	46	71

Single event analysis – SWMHYMO

The IDF parameters a, b & c are obtained from the City of Calgary Design Guidelines.

Table 2 – IDF Parameters

Return Period	a	b	c	Total 24hr Rainfall Depth from AES data (mm)
1:5 Year	353.5	2.29	0.703	51.2
1:100 Year	663.1	1.87	0.712	89.4

The CN value is the runoff curve from the USDA Soil Conservation Service 1968.

- 5 minute interval, 24 hour duration
- Peak at 30 percent of duration of storm event
- For developed grasses areas, a value of 72 was used for the curve number (CN). This corresponds with moisture condition II, urban – open space, good condition, soil group B.
- CALIB STANDHYD is a command in SWMHYMO application to simulate runoff from urban areas with impervious ratios higher than 0.20, or indirectly connected surfaces (i.e. mixed surfacing that flows overland). The user must provide the values for 'Ia' and 'N'. Rainfall losses can be simulated with either Horton's infiltration equations or modified SCS procedure or a proportional loss coefficient. In this report, modified SCS procedure was applied. Following is a description used for the runoff simulation:

- Computational Time Step (DT) = 1 or 5 minutes used in all cases, depending on the existing SWMHYMO model for external areas
- Percent Impervious XIMP - Ratio of areas directly connected to the minor system
- Percent Impervious TIMP - Ratio of total impervious areas, equal to XIMP
- Base Flows (DWF) - Zero in all cases
- CN value of grass with soil type C = 72
- Initial Abstraction (IA) - For pervious areas only
- IA = based on CN Value = $0.2 * (25400 / CN - 254)$ unless stated otherwise
- Depression Storage (DPSI) - Impervious areas only
- DPSI = 1.6 mm in all cases
- For pervious surfaces, MNP = 0.25.
- For impervious surfaces, MNI = 0.013 or 0.014
- SCP = 0 or 10.00 min
- SCI = 0 or 5.00 min

Following is a brief description of SWMHYMO computer model commands typically used for simulation of overland runoff interception regime to the minor system:

READ HYD	Command for reading a previously saved hydrograph from a text file
ADD HYD	Command for adding hydrographs
ROUTE RESERVOIR	Command for routing flow hydrographs through detention storage facilities (i.e. trap low, pond) with a storage-outflow relationship
DIVERT HYD	Operational command which can be used to split a hydrograph into two or more hydrographs
COMPUTE DUALHYD	Command for separating the major system (street flow) and minor system (pipe flow) hydrograph from a total hydrograph

SAVE HYD	Command for saving a hydrograph to file
COMPUTE VOLUME	An operational command that can be used to compute a total volume of a hydrograph

Continuous Modelling - WBSCC

In PCSWMM, the pond is modelled as a separate catchment, so the catchments are broken down as shown in **Table 3**.

Table 3 – PCSWMM Catchment Properties

Surface	Area (m ²)	% Imp	% Imp to grass
Residential	25.50	40	100
Roads	16.41	65	50
MR	3.94	5	100
Sub-total	45.85	46	75
Pond/ PUL	1.01	100	0
Pond/ MR	0.86	5	100
TOTAL	47.72	46	71

For the subsoil hydraulic infiltration rates, as mentioned above, a conservative value of 1×10^{-8} m/s is used. It is expected that this value will be increased during the next design stage.

The WBSCC general catchment characteristics are given in **Table 4** below.

Table 4 General Water Balance Spreadsheet Characteristics

Ref	Unit	Asphalt/Roof	Gravel*	Cut Grass
Depression Loss	mm	1.6	-	-
Sand	%	n/a	88	30
Silt	%	n/a	7	35
Clay	%	n/a	5	35
Media Depth	mm	n/a	50	150
Porosity	% vol	n/a	0.46	0.48
Field Capacity	% vol	n/a	0.10	0.36
Wilting Point	% vol	n/a	0.05	0.22
Sat. Hyd. Cond.	m/s	n/a	3.00E-05	1.19E-06
Sub-soil Hyd. Cond.	m/s	n/a	1.0 E-06	1.0 E-06
Ponding Depth	mm	n/a	3.2	3.2

* Characteristics from email from City of Calgary Water Resources 14th Feb 2013

Results

For the 1:100-year design storm, using SWMHYMO, the size of the bioretention area was adjusted until the discharge volume and peak flow rate matched the pre-development values. The 1:5-year design storm was then simulated to compare pre and post development values.

QHM was then used to compare annual runoff volumes and also to model theoretical water quality values. Once the results are compiled, the bioretention area can be altered to minimize the impact on the downstream properties and watercourse.

Table 5 below shows the size of bioretention are required.

Table 5 Proposed Pond

Elevation (m)	Step (m)	Area (m ²)	Stage Volume (m ³)	Total Volume (m ³)	Discharge (m ³ /s)	Notes
1297.60	0					Btm
1298.10	0.5	1,984	331	331		
1298.60	0.5	2,460	1,109	1,440		
1299.10	0.5	2,986	1,359	2,799		
1299.60	0.5	3,561	1,635	4,434		
1300.10	0.5	4,185	1,934	6,367		
1300.60	0.5	4,859	2,259	8,627	0	NWL
1301.10	0.5	5,582	2,608	11,235	0.0223	
1301.60	0.5	6,354	2,982	14,217	0.0330	
1302.10	0.5	7,176	3,380	17,597	0.0404	
1302.50	0.4	7,869	3,008	20,605	0.0455	1:5 Yr
1302.60	0.1	9,069	846	21,451	0.0467	
1303.10	0.5	15,943	6,172	27,624	0.0522	
1303.60	0.5	17,306	8,310	35,934	0.0572	
1303.80	0.2	17,864	3,517	39,451	0.0590	HWL
1304.10	0.3	18,716	5,487	44,937	0.0617	Freeboard

Total Volume Balance

Due to the high-density nature of this development, there is a challenge to reduce the unit average annual runoff (UARR) to the required 30-40 mm. There is, however, a solution would involve a second development which is under the control of the same landowner, Cochrane North.

As Cochrane North is less dense, it would be easier to implement one of the proposed LID measures there, namely, the alternative road cross-section. If we can direct 70% of the road runoff to roadside

bioswales, we can reduce the overall discharge volume significantly. This means that we can increase the discharge volume from the Hamlet. This, in turn, means that the proposed MR would be inundated even less frequently than thought and that both subdivisions would have no need for irrigation using pond water. The design is to keep all storm event up to the 1:5 year storm inside the PUL. Any green space above this elevation would be classified as Municipal Reserve and can be used as an amenity such as a public park or public recreation area, ~~s~~ in accordance with the Municipal Government Act.

Based on the above values, it is projected that the following runoff volume targets are achieved.

Table 6 Allowable Annual Runoff Volumes – WBSCC

Subdivision	Total Area (Ha)	Annual Discharge (m³)	Annual Depth (mm)
Cochrane North	108.92	37,299	34.2
Cochrane Hamlet	47.72	24,992	52.4
Total	156.92	62,291	39.8

From the 51 year results of the WBSCC, HYDSTAT was used to extrapolate the 1:5 year and 1:100 year volumes using statistical analysis. **Table 7** shows the results of the analysis.

Table 7 HYDSTAT Results

Statistical Distribution	1:5 Year Volume (m³)	1:100 Year Volume (m³)	Least-Squares Priority Order
Normal	21,209	31,411	7
Log-Normal	19,679	34,746	6
Gumbel	20,372	36,974	4
Log-Gumbel	18,782	47,373	3
Pearson III	19,848	39,332	1
Log-Pearson III	19,034	45,425	2
Log-Normal III	19,472	35,935	5
Storage Provided	20,605	39,451	

These results include a passive (below Normal Water Level) volume of 8,627 m³. In order to compare the results with the single event analysis results (SWMHYMO), this value must be subtracted.

Table 8 Pond Volumes

	1:5 Year (m ³)	1:100 Year (m ³)
SWMHYMO	9,712	23,570
HYDSTAT	11,221	30,705
Storage Provided	11,978	30,824

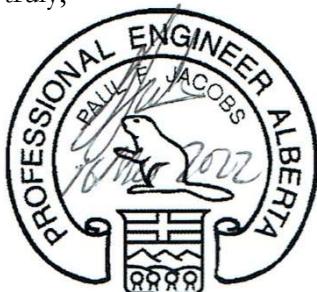
The results show that the continuous simulation gives the higher storage volume requirements and that both sets of data show that the pond has sufficient capacity.

References

- Geotechnical Assessment for the Proposed Subdivision Cochrane Lake Hamlet by Watt Consulting in January 2022USDA Soil Conservation Service 1968
- City of Calgary, “Stormwater Management & Design Manual”, Wastewater & Drainage Department, September 2011
- J.F. Sabourin and Associates Inc., “SWMHYMO Stormwater Management Hydrologic Model – User’s Manual”, May 2000 (reprinted April 2005)
- Westhoff Engineering Resources, Inc. for the City of Calgary, “User Manual for Water Balance Spreadsheet, Version 1.2,” November 2011

We trust that this report is acceptable but if you have any questions, or require further information, please do not hesitate to contact me.

Yours truly,

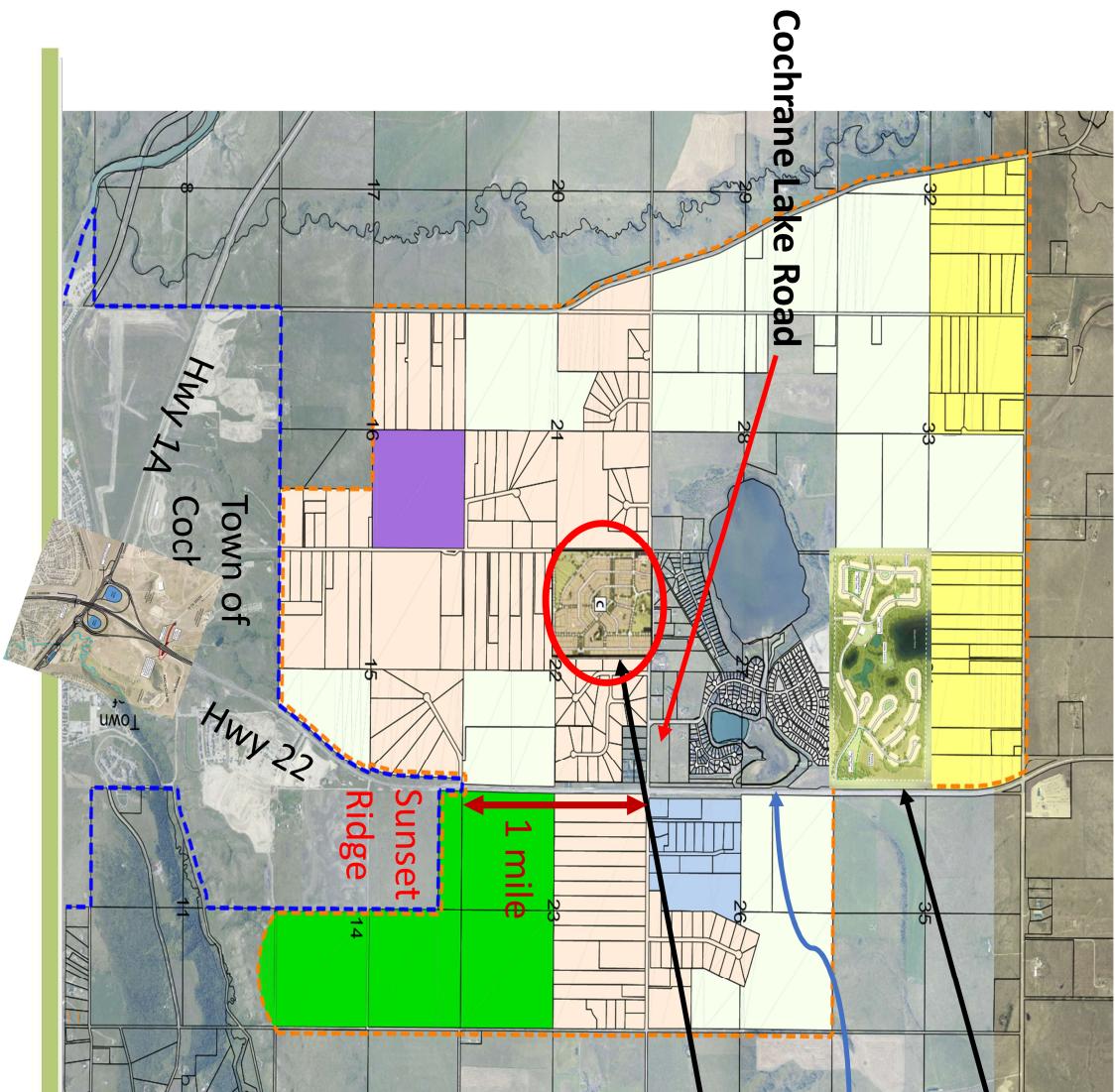


Paul Jacobs, P. Eng
 Water Management Engineer

PERMIT TO PRACTICE	
STORMWATER SOLUTIONS	
RM SIGNATURE:	
RM APEGA ID #:	071984
DATE:	16 Mar 2022
PERMIT NUMBER: P014294	
The Association of Professional Engineers and Geoscientists of Alberta (APEGA)	

Enc:

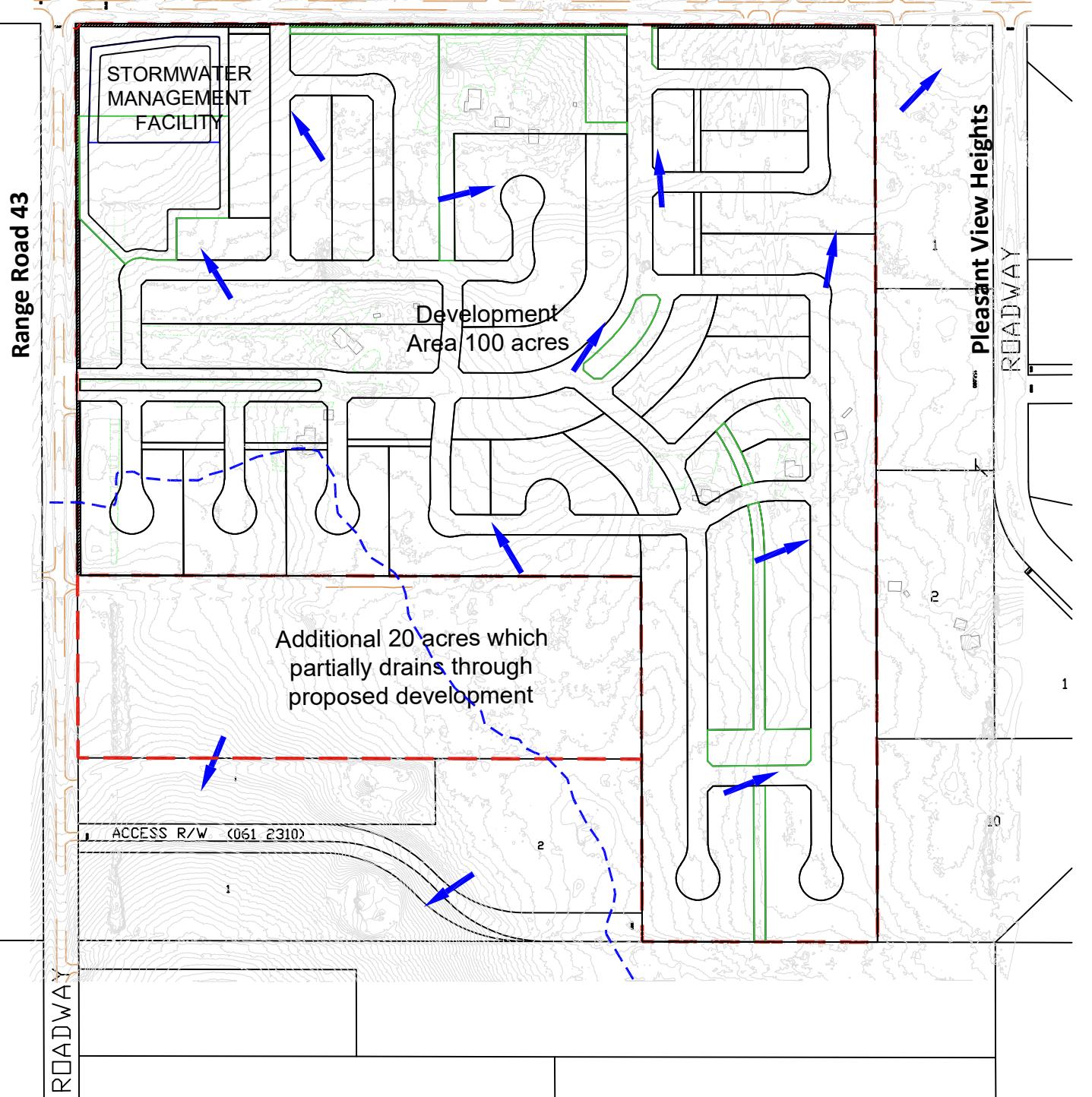
- Figure 1 – Location Plan
- Figure 2 – Site Plan
- Figure 3 – Pond Layout
- Figure 4 – Details
- SWMHYMO model files
- WBSCC results files



Stormwater Solutions	Scale N.T.S	Client: Schickedanz North Ltd Project Name: COCHRANE LAKE HAMLET PLAN NEIGHBOURHOOD C Drawing Title: Location Plan Drawing Number: Figure 1 Project Number: 0246-002
A ISSUED FOR CONCEPT PLAN 2022-03-16		



Cochrane Lake Road West



Stormwater Solutions			
A	ISSUED FOR CONCEPT PLAN	2022-03-16	
Rev.	Description		

Scale 1:5,000


 Development/
 Study Area
 Boundary


 Cochrane Lake
 Catchment
 Boundary

Client: Schickedanz North Ltd

Project Name:

COCHRANE LAKE HAMLET PLAN

Drawing Title: Site Plan

Drawing Title: Site Plan
Drawing Number: Figure 2

Drawing Number: Figure 2
Project Number: 0246-002



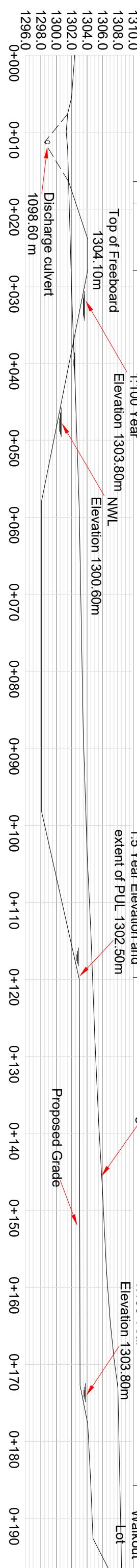
			Scale 1:1,250	Client: Schickedanz North Ltd Project Name: COCHRANE LAKE HAMLET PLAN NEIGHBOURHOOD C
	A ISSUED FOR CONCEPT PLAN	2022-03-16		Drawing Title: Pond Layout Drawing Number: Figure 3 Project Number: 0246-002
Rev. Description				



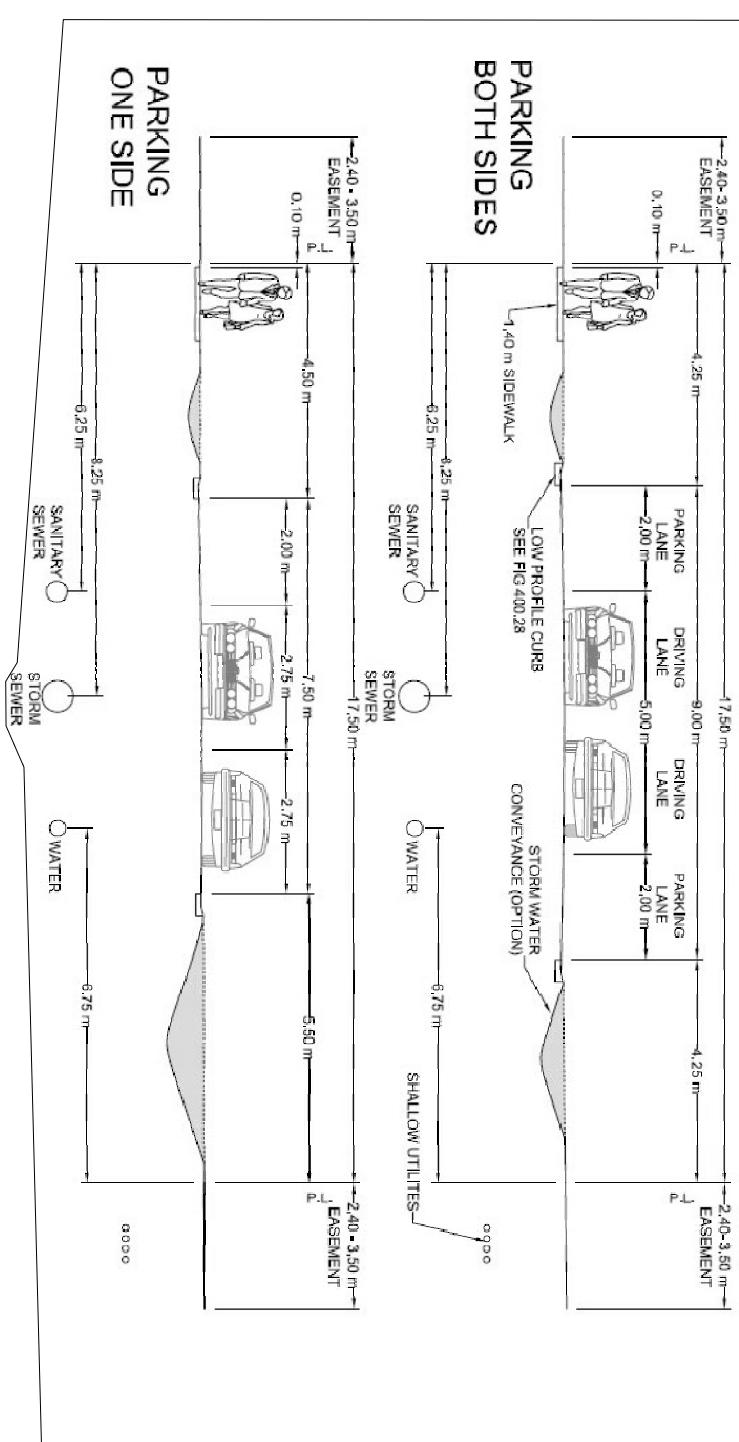
CL Cochrane
Lake Road

Highway
Widening
Line

Property
Line



CROSS-SECTION
STORMWATER MANAGEMENT FACILITY
Scale 1:500



Example Only. Design to be Completed by Engineering
Consultant

Scale as noted

Stormwater Solutions	Client: Schickedanz North Ltd Project Name: COCHRANE LAKE HAMLET PLAN NEIGHBOURHOOD C
A ISSUED FOR CONCEPT PLAN 2022-03-16	Drawing Title: Details Drawing Number: Figure 4 Project Number: 0246-002

2 Metric units

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*FILENAME: HAMC-S01.dat
*COCHRANE HAMLET - AREA C
*Mar 2022
*PFJ
*100 YEAR DESIGN STORM (24 HOUR)
*
START TZERO=[0.0], METOUT=[2]

*5 YEAR CHICAGO DESIGN STORM - Calgary
*
CHICAGO STORM IUNITS=2 (metric) TD=24.0 hrs R=0.3 SDT=5.0 min ICASE=1
A=353.5 B=0.229 C=0.703

*Prop
CALIB STANDHYD ID NHYD DT(min) AREA(ha) XIMP TIMP DWF(cms) LOSS
1 101 1.00 47.72 0.13 0.46 0 2
MODIFIED CN=72
PERVIOUS AREA: DPSP(mm) SLOPE(%) LGP(m) MNP SCP
3.2 2.0 20 0.250 0
IMPERVIOUS AREA: DPSI(mm) SLOPE(%) LGI(m) MNI SCI
1.6 2.0 500 0.013 0
END = -1

*Pond
ROUTE RESERVOIR ID=2 NHYD=201 IDIN=1 DT=1.0 min
DISCH(cms) STORAGE(ha m)
0 0
0.0233 0.2608
0.0330 0.5590
0.0404 0.8871
0.0455 1.1978
0.0467 1.2845
0.0522 1.8997
0.0572 2.7307
0.0590 3.0824
0.0617 3.8569
-1 -1
IDovf=3, NHYDovf=301

*100 YEAR CHICAGO DESIGN STORM - Calgary
*
CHICAGO STORM IUNITS=2 (metric) TD=24.0 hrs R=0.3 SDT=5.0 min ICASE=1
A=663.1 B=1.87 C=0.712

*Prop
CALIB STANDHYD ID NHYD DT(min) AREA(ha) XIMP TIMP DWF(cms) LOSS
1 101 1.00 47.72 0.13 0.46 0 2
MODIFIED CN=72
PERVIOUS AREA: DPSP(mm) SLOPE(%) LGP(m) MNP SCP
3.2 2.0 20 0.250 0
IMPERVIOUS AREA: DPSI(mm) SLOPE(%) LGI(m) MNI SCI
1.6 2.0 500 0.013 0
END = -1

*Pond
ROUTE RESERVOIR ID=2 NHYD=201 IDIN=1 DT=1.0 min
DISCH(cms) STORAGE(ha m)
0 0
0.0233 0.2608
0.0330 0.5590
0.0404 0.8871

0.0455 1.1978
0.0467 1.2845
0.0522 1.8997
0.0572 2.7307
0.0590 3.0824
0.0617 3.8569
-1 -1
IDovf=3, NHYDovf=301

*25 YEAR CHICAGO DESIGN STORM - Calgary
*
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A=522.6 B=1.960 C=0.709

*Prop
CALIB STANDHYD ID NHYD DT(min) AREA(ha) XIMP TIMP DWF(cms) LOSS
1 101 1.00 47.72 0.31 0.46 0 2
MODIFIED CN=72
PERVIOUS AREA: DPSP(mm) SLOPE(%) LGP(m) MNP SCP
3.2 2.0 20 0.250 0
IMPERVIOUS AREA: DPSI(mm) SLOPE(%) LGI(m) MNI SCI
1.6 2.0 500 0.013 0
END = -1

*Pond
ROUTE RESERVOIR ID=2 NHYD=201 IDIN=1 DT=1.0 min
DISCH(cms) STORAGE(ha m)
0 0
0.0233 0.2608
0.0330 0.5590
0.0404 0.8871
0.0455 1.1978
0.0467 1.2845
0.0522 1.8997
0.0572 2.7307
0.0590 3.0824
0.0617 3.8569
-1 -1
IDovf=3, NHYDovf=301

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9 9 9 9 # 3826891
StormWater Management HYdrologic Model 999 999 =====
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***** A single event and continuous hydrologic simulation model *****
***** based on the principles of HYMO and its successors *****
***** OTTHYMO-83 and OTTHYMO-89. *****
***** Distributed by: J.F. Sabourin and Associates Inc. *****
***** Ottawa, Ontario: (613) 836-3884 *****
***** Gatineau, Quebec: (819) 243-6858 *****
***** E-Mail: swmhymo@jfsa.Com *****
*****
+++++ Licensed user: Stormwater Solutions Inc. ++++++
++++++ Calgary SERIAL#:3826891 ++++++
+++++
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* Input filename: C:\DATA\SWMHYMO\HAMC-S01.dat *
* Output filename: C:\DATA\SWMHYMO\HAMC-S01.out *
* Summary filename: C:\DATA\SWMHYMO\HAMC-S01.sum *
* User comments:
* 1: _____ *
* 2: _____ *
* 3: _____ *
-----
001:0001-----
*****
*FILENAME: HAMC-S01.dat
*COCHRANE HAMLET - AREA C
*Mar 2022
*PFJ
*100 YEAR DESIGN STORM (24 HOUR)
* -----
| START | Project dir.: C:\DATA\SWMHYMO\
----- Rainfall dir.: C:\DATA\SWMHYMO\
TZERO = .00 hrs on 0
METOUT= 2 (output = METRIC)
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NRUN = 001
NSTORM= 0
```

```
001:0002-----
```

```
***** 5 YEAR CHICAGO DESIGN STORM - Calgary
```

```
*
| CHICAGO STORM | IDF curve parameters: A= 353.500
| Ptotal= 51.08 mm | B= .229
C= .703
```

```
used in: INTENSITY = A / (t + B)^C
```

```
Duration of storm = 24.00 hrs
```

```
Storm time step = 5.00 min
```

```
Time to peak ratio = .30
```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
.08	.641	6.08	2.425	12.08	1.497	18.08	.855
.17	.646	6.17	2.568	12.17	1.480	18.17	.851
.25	.652	6.25	2.734	12.25	1.463	18.25	.846
.33	.657	6.33	2.928	12.33	1.446	18.33	.842
.42	.663	6.42	3.160	12.42	1.430	18.42	.837
.50	.669	6.50	3.441	12.50	1.414	18.50	.833
.58	.675	6.58	3.792	12.58	1.399	18.58	.829
.67	.681	6.67	4.245	12.67	1.384	18.67	.825
.75	.687	6.75	4.856	12.75	1.370	18.75	.820
.83	.693	6.83	5.737	12.83	1.355	18.83	.816
.92	.700	6.92	7.148	12.92	1.342	18.92	.812
1.00	.706	7.00	9.892	13.00	1.328	19.00	.808
1.08	.713	7.08	19.340	13.08	1.315	19.08	.804
1.17	.720	7.17	110.495	13.17	1.302	19.17	.800
1.25	.727	7.25	25.185	13.25	1.290	19.25	.796
1.33	.735	7.33	15.672	13.33	1.277	19.33	.793
1.42	.742	7.42	11.886	13.42	1.265	19.42	.789
1.50	.750	7.50	9.759	13.50	1.254	19.50	.785
1.58	.758	7.58	8.370	13.58	1.242	19.58	.781
1.67	.766	7.67	7.380	13.67	1.231	19.67	.778
1.75	.774	7.75	6.634	13.75	1.220	19.75	.774
1.83	.783	7.83	6.048	13.83	1.209	19.83	.771
1.92	.791	7.92	5.573	13.92	1.199	19.92	.767
2.00	.800	8.00	5.180	14.00	1.189	20.00	.763
2.08	.810	8.08	4.847	14.08	1.178	20.08	.760
2.17	.819	8.17	4.562	14.17	1.169	20.17	.757
2.25	.829	8.25	4.315	14.25	1.159	20.25	.753
2.33	.839	8.33	4.098	14.33	1.149	20.33	.750
2.42	.849	8.42	3.905	14.42	1.140	20.42	.747
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2.58	.871	8.58	3.579	14.58	1.122	20.58	.740
2.67	.882	8.67	3.439	14.67	1.113	20.67	.737
2.75	.894	8.75	3.311	14.75	1.105	20.75	.734
2.83	.906	8.83	3.195	14.83	1.096	20.83	.730
2.92	.919	8.92	3.087	14.92	1.088	20.92	.727
3.00	.932	9.00	2.989	15.00	1.080	21.00	.724
3.08	.945	9.08	2.897	15.08	1.072	21.08	.721
3.17	.959	9.17	2.812	15.17	1.064	21.17	.718
3.25	.973	9.25	2.733	15.25	1.056	21.25	.715
3.33	.988	9.33	2.659	15.33	1.049	21.33	.712
3.42	1.004	9.42	2.590	15.42	1.041	21.42	.709
3.50	1.020	9.50	2.525	15.50	1.034	21.50	.706
3.58	1.036	9.58	2.463	15.58	1.027	21.58	.704
3.67	1.054	9.67	2.406	15.67	1.020	21.67	.701

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3.75	1.072		9.75	2.351		15.75	1.013		21.75	.698
3.83	1.091		9.83	2.299		15.83	1.006		21.83	.695
3.92	1.110		9.92	2.250		15.92	.999		21.92	.692
4.00	1.131		10.00	2.204		16.00	.992		22.00	.690
4.08	1.153		10.08	2.159		16.08	.986		22.08	.687
4.17	1.175		10.17	2.117		16.17	.980		22.17	.684
4.25	1.199		10.25	2.077		16.25	.973		22.25	.682
4.33	1.224		10.33	2.038		16.33	.967		22.33	.679
4.42	1.250		10.42	2.002		16.42	.961		22.42	.676
4.50	1.277		10.50	1.966		16.50	.955		22.50	.674
4.58	1.306		10.58	1.933		16.58	.949		22.58	.671
4.67	1.337		10.67	1.900		16.67	.943		22.67	.669
4.75	1.370		10.75	1.869		16.75	.937		22.75	.666
4.83	1.404		10.83	1.839		16.83	.932		22.83	.664
4.92	1.441		10.92	1.811		16.92	.926		22.92	.661
5.00	1.480		11.00	1.783		17.00	.920		23.00	.659
5.08	1.522		11.08	1.756		17.08	.915		23.08	.656
5.17	1.566		11.17	1.731		17.17	.910		23.17	.654
5.25	1.614		11.25	1.706		17.25	.904		23.25	.652
5.33	1.666		11.33	1.682		17.33	.899		23.33	.649
5.42	1.722		11.42	1.658		17.42	.894		23.42	.647
5.50	1.783		11.50	1.636		17.50	.889		23.50	.644
5.58	1.849		11.58	1.614		17.58	.884		23.58	.642
5.67	1.922		11.67	1.593		17.67	.879		23.67	.640
5.75	2.002		11.75	1.573		17.75	.874		23.75	.638
5.83	2.090		11.83	1.553		17.83	.869		23.83	.635
5.92	2.189		11.92	1.534		17.92	.865		23.92	.633
6.00	2.300		12.00	1.515		18.00	.860		24.00	.631

001:0003-----

*Prop

CALIB STANDHYD	Area (ha)=	47.72
01:000101 DT= 1.00	Total Imp(%)=	46.00 Dir. Conn.(%)= 13.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	21.95	25.77
Dep. Storage (mm)=	1.60	3.20
Average Slope (%)=	2.00	2.00
Length (m)=	500.00	20.00
Mannings n =	.013	.250
Max.eff.Inten.(mm/hr)=	110.49	35.68
over (min)	5.00	12.00
Storage Coeff. (min)=	5.24 (ii)	12.27 (ii)
Unit Hyd. Tpeak (min)=	5.00	12.00
Unit Hyd. peak (cms)=	.22	.09

TOTALS

PEAK FLOW (cms)=	1.11	1.56	2.158 (iii)
TIME TO PEAK (hrs)=	7.22	7.33	7.300
RUNOFF VOLUME (mm)=	49.47	21.82	25.423
TOTAL RAINFALL (mm)=	51.08	51.08	51.076
RUNOFF COEFFICIENT =	.97	.43	.498

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0004-----

*ROUTING RESULTS AREA QPEAK TPEAK R.V.
----- (ha) (cms) (hrs) (mm)
INFLOW >01: (000101) 47.72 2.158 7.300 25.423
OUTFLOW<02: (000201) 47.72 .042 24.167 25.421
OVERFLOW<03: (000301) .00 .000 .000 .000
TOTAL NUMBER OF SIMULATED OVERFLOWS = 0
CUMULATIVE TIME OF OVERFLOWS (hours)= .00
PERCENTAGE OF TIME OVERFLOWING (%)= .00

PEAK FLOW REDUCTION [Qout/Qin] (%)= 1.936
TIME SHIFT OF PEAK FLOW (min)= 1012.00
MAXIMUM STORAGE USED (ha.m.)=.9712E+00

001:0005-----

*100 YEAR CHICAGO DESIGN STORM - Calgary
*

| CHICAGO STORM | IDF curve parameters: A= 663.100
| Pttotal= 89.67 mm | B= 1.870
C= .712
used in: INTENSITY = A / (t + B)^C
Duration of storm = 24.00 hrs
Storm time step = 5.00 min
Time to peak ratio = .30

TIME hrs	RAIN mm/hr						
.08	1.094	6.08	4.259	12.08	2.597	18.08	1.467
.17	1.103	6.17	4.519	12.17	2.566	18.17	1.460
.25	1.113	6.25	4.821	12.25	2.536	18.25	1.452
.33	1.122	6.33	5.176	12.33	2.506	18.33	1.444
.42	1.132	6.42	5.601	12.42	2.478	18.42	1.436
.50	1.143	6.50	6.120	12.50	2.450	18.50	1.429
.58	1.153	6.58	6.773	12.58	2.423	18.58	1.421
.67	1.163	6.67	7.624	12.67	2.396	18.67	1.414
.75	1.174	6.75	8.785	12.75	2.371	18.75	1.407
.83	1.185	6.83	10.488	12.83	2.346	18.83	1.399
.92	1.197	6.92	13.283	12.92	2.321	18.92	1.392
1.00	1.208	7.00	18.961	13.00	2.297	19.00	1.385

3/16/2022

1.08	1.220		7.08	40.516		13.08	2.274		19.08	1.378
1.17	1.232		7.17	168.138		13.17	2.252		19.17	1.372
1.25	1.245		7.25	54.372		13.25	2.229		19.25	1.365
1.33	1.257		7.33	31.748		13.33	2.208		19.33	1.358
1.42	1.270		7.42	23.236		13.42	2.187		19.42	1.352
1.50	1.284		7.50	18.660		13.50	2.166		19.50	1.345
1.58	1.297		7.58	15.763		13.58	2.146		19.58	1.339
1.67	1.311		7.67	13.746		13.67	2.126		19.67	1.332
1.75	1.326		7.75	12.251		13.75	2.107		19.75	1.326
1.83	1.341		7.83	11.093		13.83	2.088		19.83	1.320
1.92	1.356		7.92	10.166		13.92	2.069		19.92	1.313
2.00	1.372		8.00	9.405		14.00	2.051		20.00	1.307
2.08	1.388		8.08	8.768		14.08	2.034		20.08	1.301
2.17	1.404		8.17	8.225		14.17	2.016		20.17	1.295
2.25	1.421		8.25	7.756		14.25	1.999		20.25	1.289
2.33	1.439		8.33	7.346		14.33	1.983		20.33	1.284
2.42	1.457		8.42	6.985		14.42	1.966		20.42	1.278
2.50	1.476		8.50	6.664		14.50	1.950		20.50	1.272
2.58	1.495		8.58	6.376		14.58	1.935		20.58	1.266
2.67	1.515		8.67	6.116		14.67	1.919		20.67	1.261
2.75	1.535		8.75	5.880		14.75	1.904		20.75	1.255
2.83	1.556		8.83	5.665		14.83	1.889		20.83	1.250
2.92	1.578		8.92	5.468		14.92	1.875		20.92	1.244
3.00	1.601		9.00	5.287		15.00	1.860		21.00	1.239
3.08	1.624		9.08	5.119		15.08	1.846		21.08	1.234
3.17	1.648		9.17	4.964		15.17	1.833		21.17	1.229
3.25	1.674		9.25	4.819		15.25	1.819		21.25	1.223
3.33	1.700		9.33	4.684		15.33	1.806		21.33	1.218
3.42	1.727		9.42	4.558		15.42	1.793		21.42	1.213
3.50	1.755		9.50	4.440		15.50	1.780		21.50	1.208
3.58	1.784		9.58	4.329		15.58	1.767		21.58	1.203
3.67	1.815		9.67	4.224		15.67	1.755		21.67	1.198
3.75	1.846		9.75	4.125		15.75	1.743		21.75	1.193
3.83	1.880		9.83	4.032		15.83	1.731		21.83	1.188
3.92	1.914		9.92	3.943		15.92	1.719		21.92	1.184
4.00	1.950		10.00	3.859		16.00	1.707		22.00	1.179
4.08	1.988		10.08	3.780		16.08	1.696		22.08	1.174
4.17	2.028		10.17	3.704		16.17	1.685		22.17	1.170
4.25	2.070		10.25	3.631		16.25	1.673		22.25	1.165
4.33	2.113		10.33	3.562		16.33	1.663		22.33	1.160
4.42	2.159		10.42	3.496		16.42	1.652		22.42	1.156
4.50	2.208		10.50	3.433		16.50	1.641		22.50	1.151
4.58	2.259		10.58	3.373		16.58	1.631		22.58	1.147
4.67	2.313		10.67	3.315		16.67	1.621		22.67	1.143
4.75	2.371		10.75	3.259		16.75	1.611		22.75	1.138
4.83	2.432		10.83	3.206		16.83	1.601		22.83	1.134
4.92	2.497		10.92	3.154		16.92	1.591		22.92	1.130
5.00	2.566		11.00	3.105		17.00	1.581		23.00	1.125
5.08	2.640		11.08	3.057		17.08	1.572		23.08	1.121
5.17	2.719		11.17	3.011		17.17	1.562		23.17	1.117
5.25	2.805		11.25	2.967		17.25	1.553		23.25	1.113
5.33	2.897		11.33	2.924		17.33	1.544		23.33	1.109
5.42	2.997		11.42	2.883		17.42	1.535		23.42	1.105
5.50	3.105		11.50	2.843		17.50	1.526		23.50	1.101
5.58	3.224		11.58	2.805		17.58	1.517		23.58	1.097
5.67	3.354		11.67	2.767		17.67	1.509		23.67	1.093
5.75	3.497		11.75	2.731		17.75	1.500		23.75	1.089
5.83	3.656		11.83	2.696		17.83	1.492		23.83	1.085
5.92	3.833		11.92	2.662		17.92	1.484		23.92	1.081
6.00	4.033		12.00	2.629		18.00	1.476		24.00	1.077

PEAK FLOW REDUCTION [Qout/Qin] (%) = .934
 TIME SHIFT OF PEAK FLOW (min) = 1015.00
 MAXIMUM STORAGE USED (ha.m.) = .2357E+01

001:0008-----

 *25 YEAR CHICAGO DESIGN STORM - Calgary
 *

 | CHICAGO STORM | IDF curve parameters: A= 522.600
 | Ptotal= 72.22 mm | B= 1.960
 | | C= .709

 used in: INTENSITY = A / (t + B)^C
 Duration of storm = 24.00 hrs
 Storm time step = 5.00 min
 Time to peak ratio = .30
 TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
 hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
 .08 .890 | 6.08 3.448 | 12.08 2.106 | 18.08 1.193
 .17 .898 | 6.17 3.658 | 12.17 2.081 | 18.17 1.186
 .25 .906 | 6.25 3.901 | 12.25 2.057 | 18.25 1.180
 .33 .914 | 6.33 4.187 | 12.33 2.033 | 18.33 1.174
 .42 .922 | 6.42 4.530 | 12.42 2.010 | 18.42 1.168
 .50 .930 | 6.50 4.949 | 12.50 1.987 | 18.50 1.162
 .58 .938 | 6.58 5.475 | 12.58 1.966 | 18.58 1.156
 .67 .947 | 6.67 6.160 | 12.67 1.944 | 18.67 1.150
 .75 .955 | 6.75 7.095 | 12.75 1.924 | 18.75 1.144
 .83 .964 | 6.83 8.466 | 12.83 1.903 | 18.83 1.138
 .92 .974 | 6.92 10.716 | 12.92 1.884 | 18.92 1.132
 1.00 .983 | 7.00 15.283 | 13.00 1.864 | 19.00 1.126
 1.08 .992 | 7.08 32.555 | 13.08 1.846 | 19.08 1.121
 1.17 1.002 | 7.17 132.056 | 13.17 1.827 | 19.17 1.115
 1.25 1.012 | 7.25 43.630 | 13.25 1.809 | 19.25 1.110
 1.33 1.023 | 7.33 25.552 | 13.33 1.792 | 19.33 1.104
 1.42 1.033 | 7.42 18.719 | 13.42 1.775 | 19.42 1.099
 1.50 1.044 | 7.50 15.041 | 13.50 1.758 | 19.50 1.094
 1.58 1.055 | 7.58 12.711 | 13.58 1.742 | 19.58 1.089
 1.67 1.067 | 7.67 11.088 | 13.67 1.726 | 19.67 1.083
 1.75 1.078 | 7.75 9.885 | 13.75 1.710 | 19.75 1.078
 1.83 1.090 | 7.83 8.953 | 13.83 1.695 | 19.83 1.073
 1.92 1.103 | 7.92 8.207 | 13.92 1.680 | 19.92 1.068
 2.00 1.115 | 8.00 7.595 | 14.00 1.665 | 20.00 1.063
 2.08 1.128 | 8.08 7.081 | 14.08 1.651 | 20.08 1.058
 2.17 1.142 | 8.17 6.644 | 14.17 1.637 | 20.17 1.054
 2.25 1.156 | 8.25 6.266 | 14.25 1.623 | 20.25 1.049
 2.33 1.170 | 8.33 5.936 | 14.33 1.610 | 20.33 1.044
 2.42 1.184 | 8.42 5.646 | 14.42 1.597 | 20.42 1.039
 2.50 1.199 | 8.50 5.387 | 14.50 1.584 | 20.50 1.035
 2.58 1.215 | 8.58 5.155 | 14.58 1.571 | 20.58 1.030
 2.67 1.231 | 8.67 4.945 | 14.67 1.558 | 20.67 1.026
 2.75 1.248 | 8.75 4.795 | 14.75 1.546 | 20.75 1.021
 2.83 1.265 | 8.83 4.582 | 14.83 1.534 | 20.83 1.017
 2.92 1.282 | 8.92 4.423 | 14.92 1.522 | 20.92 1.012
 3.00 1.301 | 9.00 4.277 | 15.00 1.511 | 21.00 1.008
 3.08 1.320 | 9.08 4.141 | 15.08 1.500 | 21.08 1.004
 3.17 1.339 | 9.17 4.016 | 15.17 1.488 | 21.17 .999
 3.25 1.360 | 9.25 3.900 | 15.25 1.477 | 21.25 .995
 3.33 1.381 | 9.33 3.791 | 15.33 1.467 | 21.33 .991
 3.42 1.403 | 9.42 3.689 | 15.42 1.456 | 21.42 .987
 3.50 1.426 | 9.50 3.594 | 15.50 1.446 | 21.50 .983
 3.58 1.449 | 9.58 3.504 | 15.58 1.436 | 21.58 .979
 3.67 1.474 | 9.67 3.420 | 15.67 1.426 | 21.67 .975
 3.75 1.500 | 9.75 3.340 | 15.75 1.416 | 21.75 .971

3.83	1.526		9.83	3.265		15.83	1.406		21.83	.967
3.92	1.554		9.92	3.193		15.92	1.396		21.92	.963
4.00	1.584		10.00	3.126		16.00	1.387		22.00	.959
4.08	1.614		10.08	3.061		16.08	1.378		22.08	.955
4.17	1.646		10.17	3.000		16.17	1.369		22.17	.952
4.25	1.680		10.25	2.942		16.25	1.360		22.25	.948
4.33	1.715		10.33	2.886		16.33	1.351		22.33	.944
4.42	1.753		10.42	2.833		16.42	1.342		22.42	.941
4.50	1.792		10.50	2.782		16.50	1.334		22.50	.937
4.58	1.833		10.58	2.733		16.58	1.325		22.58	.933
4.67	1.877		10.67	2.686		16.67	1.317		22.67	.930
4.75	1.924		10.75	2.641		16.75	1.309		22.75	.926
4.83	1.973		10.83	2.598		16.83	1.301		22.83	.923
4.92	2.025		10.92	2.557		16.92	1.293		22.92	.919
5.00	2.081		11.00	2.517		17.00	1.285		23.00	.916
5.08	2.141		11.08	2.478		17.08	1.277		23.08	.912
5.17	2.205		11.17	2.441		17.17	1.270		23.17	.909
5.25	2.274		11.25	2.405		17.25	1.262		23.25	.906
5.33	2.349		11.33	2.371		17.33	1.255		23.33	.902
5.42	2.429		11.42	2.338		17.42	1.248		23.42	.899
5.50	2.517		11.50	2.305		17.50	1.240		23.50	.896
5.58	2.613		11.58	2.274		17.58	1.233		23.58	.892
5.67	2.717		11.67	2.244		17.67	1.226		23.67	.889
5.75	2.833		11.75	2.215		17.75	1.220		23.75	.886
5.83	2.961		11.83	2.186		17.83	1.213		23.83	.883
5.92	3.104		11.92	2.159		17.92	1.206		23.92	.880
6.00	3.266		12.00	2.132		18.00	1.199		24.00	.877

001:0009-----

 *Prop-----
 | CALIB STANDHYD | Area (ha)= 47.72
 | 01:000101 DT= 1.00 | Total Imp(%)= 46.00 Dir. Conn.(%)= 31.00-----
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 21.95 25.77
 Dep. Storage (mm)= 1.60 3.20
 Average Slope (%)= 2.00 2.00
 Length (m)= 500.00 20.00
 Mannings n = .013 .250

Max.eff.Inten.(mm/hr)= 132.06 42.67
 over (min) 5.00 11.00
 Storage Coeff. (min)= 4.88 (ii) 11.42 (ii)
 Unit Hyd. Tpeak (min)= 5.00 11.00
 Unit Hyd. peak (cms)= .23 .10

TOTALS
 PEAK FLOW (cms)= 3.43 1.87 4.594 (iii)
 TIME TO PEAK (hrs)= 7.22 7.33 7.233
 RUNOFF VOLUME (mm)= 70.61 33.05 44.706
 TOTAL RAINFALL (mm)= 72.22 72.22 72.224
 RUNOFF COEFFICIENT = .98 .46 .619

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

001:0010-----

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*Pond

-----| ROUTE RESERVOIR | Requested routing time step = 1.0 min.

| IN>01: (000101) |

| OUT<02: (000201) |

===== OUTLFOW STORAGE TABLE =====

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
.000	.0000E+00	.047	.1285E+01
.023	.2608E+00	.052	.1900E+01
.033	.5590E+00	.057	.2731E+01
.040	.8871E+00	.059	.3082E+01
.045	.1198E+01	.062	.3857E+01

ROUTING RESULTS AREA QPEAK TPEAK R.V.

----- (ha) (cms) (hrs) (mm)

INFLOW >01: (000101) 47.72 4.594 7.233 44.706

OUTFLOW<02: (000201) 47.72 .051 24.150 44.703

OVERFLOW<03: (000301) .00 .000 .000 .000

TOTAL NUMBER OF SIMULATED OVERFLOWS = 0

CUMULATIVE TIME OF OVERFLOWS (hours)= .00

PERCENTAGE OF TIME OVERFLOWING (%)= .00

PEAK FLOW REDUCTION [Qout/Qin] (%)= 1.121

TIME SHIFT OF PEAK FLOW (min)= 1015.00

MAXIMUM STORAGE USED (ha.m.)=.1821E+01

001:0011-----

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FINISH

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WARNINGS / ERRORS / NOTES

----- Simulation ended on 2022-03-15 at 12:13:38

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WBSCC

Water Balance Spreadsheet for the City of Calgary Version 1.2

PROJECT SUMMARY SHEET

Project Name:

Area C = 100 acres

Project Description:

Slightly bigger pond

Location:

NOV 2021

Designed by:

PAUL JACOBS

Company Name:

STORMWATER SOLUTIONS

Reviewed by:

The City of Calgary Water Resources

Water Balance Spreadsheet for the City of Calgary - Version 1.2 - November 2011

WBSCC - PROJECT DATA SHEET - Environmental Information

Minimum Temperature to Trigger Runoff (°C)	0
Sublimation Losses (%)	0
Precipitation Multiplication Factor (% Decrease)	0

Month	Is Winter or Summer?	Crop Water Requirement (mm/month)			
		KENTUCKY BLUE GRASS	SAGE BRUSH	Unnamed 1	Unnamed 2
January	Winter	0	0	0	0
February	Winter	0	0	0	0
March	Winter	0	0	0	0
April	Summer	0	0	0	0
May	Summer	0	50	0	0
June	Summer	0	50	0	0
July	Summer	0	60	0	0
August	Summer	0	50	0	0
September	Summer	0	50	0	0
October	Summer	0	20	0	0
November	Winter	0	0	0	0
December	Winter	0	0	0	0

Catchment Area Data

Sub-Catchment	Description of Sub-catchment Use	Area (ha)
Sub-Catchment 1	ALL	45.8487
Sub-Catchment 2		
Sub-Catchment 3		
Sub-Catchment 4		
Sub-Catchment 5		
Total		45.8487

Pond Area Data

Pond	Description of Pond	Pond Area (m ²)
Pond 1	Pond	18716
Pond 2		0

The City of Calgary Water Resources

Water Balance Spreadsheet for the City of Calgary - Version 1.2 - November 2011

WBSCC - PROJECT DATA SHEET - Sub-Catchment 1: Parameters, Runoff Allocation

Usage: ALL

Sub-catchment Parameters	Cover Type					
	Impervious Surface	Pervious Surface	Absorbent Landscaping	Green Roof Media	Bioretention/ Bioswale Medium	Unassigned Area
Area (Total: 45.8487) (ha)	21.0634	24.7853	0	0	0	0
Depression Loss (mm)	1.6					
Soil Type: Sand		30	30	0	88	
Silt		35	35	0	7	
Clay		35	35	0	5	
Custom						
Unassigned		0	0	100	0	
Soil or Media Depth (mm)		300	50	200	300	
Porosity		0.48	0.46	0.512	0.46	
Field Capacity		0.36	0.1	0.132	0.1	
Wilting Point		0.22	0.05	0.057	0.05	
Saturated Hydraulic Conductivity (m/s)		1.19E-06	3.00E-05	2.50E-05	3.00E-05	
Sub-soil Hydraulic Conductivity (m/s)		1.00E-08	1.00E-08		1.00E-08	
Ponding Depth (mm)		3.2	0	0	0	
Inv. Slope of Log. Tension Moisture Curve		7.75	5.51	4.55	5.51	
Subdrain Invert (above bottom of media) (mm)					0	
Subdrain Capacity (m ³ /s)					0	

% of Runoff Allocated To:	Runoff Allocated from Cover Type/ Facility:						
	Impervious Surface	Pervious Surface	Absorbent Landscaping	Green Roof Media	Bioretention/ Bioswale Media	Storage/ Reuse Tank	Discharge
Pervious Surface	74.7			0			
Absorbent Landscaping	0	0		0			
Green Roof Media	0						
Storage/ Reuse Tank	0	0	0	0			
Bioretention/Bioswale Media	0	0	0	0			
Discharge	25.3	100	100	100	100	100	
Pond 1/Pond 2							POND #1

The City of Calgary Water Resources

Water Balance Spreadsheet for the City of Calgary - Version 1.2 - November 2011

WBSCC - PROJECT DATA SHEET - Pond 1: Parameters, Elevation-Area-Discharge-Volume Relationship

Pond 1 Params	Values
Base Elevation (m)	1297.60
Starting Water Elevation (m)	1300.50
Starting Discharge Elevation (UNWL) (m)	1300.60
High Water Level (HWL) (m)	1304.10
Lower Normal Water Level (LNWL) (m)	1298.60
Seepage Rate (mm/hr)	0.00
Discharge and Overflow Routed to:	OUTFALL

Pond 1 Pertinent Volumes (m ³)	Values
Volume at Base Elevation	0
Volume at Stating Water Elevation	8175
Volume at LNWL	1440
Volume at UNWL	8627
Volume at HWL	44937

Pond 1 Bed Soil Parameters	
Soil Type: Sand	30
Silt	35
Clay	35
Custom	
Unassigned	0
Soil or Media Depth (mm)	150
Porosity	0.48
Field Capacity	0.36
Wilting Point	0.22
Saturated Hydraulic Conductivity (m/s)	1.19E-06
Sub-soil Hydraulic Conductivity (m/s)	1.00E-08
Ponding Depth (mm)	0
Inv. Slope of Log. Tension Moisture Curve	7.75

Elevation (m)	Area (m ²)	Discharge (m ³ /s)
1297.60	0	0
1298.10	1,984	0
1298.60	2,460	0
1299.10	2,986	0
1299.60	3,561	0
1300.10	4,185	0
1300.60	4,859	0
1301.10	5,582	0.0233
1301.60	6,354	0.033
1302.10	7,176	0.0404
1302.50	7,869	0.0455
1302.60	9,068	0.0467
1303.10	15,943	0.0522
1303.60	17,306	0.0572
1303.80	17,864	0.059
1304.10	18,716	0.0617
1304.10	18,716	0.0617
1304.10	18,716	0.0617
1304.10	18,716	0.0617
1304.10	18,716	0.0617
1304.10	18,716	0.0617

SUBCATCHMENT 1	(mm)	(m3)	SUBCATCHMENT 1	(m3)
TOTAL MSC PRECIPITATION	20897.0	9581002.8	OVERALL WATER BALANCE OVER 51 YEARS	
AVERAGE PRECIPITATION	409.7		TOTAL MSC PRECIPITATION	9581002.8
MEDIAN PRECIPITATION	404.7		TOTAL EXTERNAL RUNON	0.0
TOTAL RUNOFF (INCLUDING SUBDRAIN)	2529.0	1159529.5	TOTAL RUNOFF (INCLUDING SUBDRAIN)	1159529.5
% OF RAINFALL AS RUNOFF	12.1		TOTAL EVAPORATION IMPERVIOUS AREAS	1060247.3
AVERAGE RUNOFF (INCLUDING SUBDRAIN)	49.6	22735.9	TOTAL EVAPOTRANSP PERVERIOUS AREAS	6533876.3
MEDIAN RUNOFF (INCLUDING SUBDRAIN)	42.3	19377.4	TOTAL RECHARGE FROM PONDS TO STORAGE TANK	0.0
TOTAL IRRIGATION DEMAND	0.0	0.0	TOTAL PERCOLATION	821150.5
MAXIMUM RUNOFF (ANY TIMESTEP)	57.9	26565.5	TOTAL SUBLIMATION LOSSES	0.0
AVERAGE EVAPORATION	279.4	128115.2	SNOW PACK AT THE END OF SIMULATION	3026.0
AVERAGE PERCOLATION	35.1	16101.0	TANK WATER BALANCE	0.0
TOTAL RUNOFF + EVAP + PERCOLATION	364.1	166952.1	WATER BALANCE	-28.8
			CONTINUITY ERROR	0.0
SC1: IMPERVIOUS AREA	(mm)	(m3)	SC1: PERVERIOUS AREA	(mm)
TOTAL MSC PRECIPITATION	20897.0	4401618.7	TOTAL MSC PRECIPITATION	20897.0
TOTAL RUNOFF	15856.8	3339981.2	TOTAL RUNOFF	1269.0
% OF RAINFALL AS RUNOFF	75.9		% OF RAINFALL AS RUNOFF	6.1
AVERAGE RUNOFF	310.9	65489.8	AVERAGE RUNOFF	24.9
MEDIAN RUNOFF	307.0	64664.6	MEDIAN RUNOFF	11.9
MAXIMUM RUNOFF (ANY TIMESTEP)	91.0	19167.7	MAXIMUM RUNOFF (ANY TIMESTEP)	87.6
TOTAL RUNON	0.0	0.0	TOTAL IRRIGATION DEMAND	0.0
TOTAL DEP STORAGE (EVAPORATION LOSS)	5033.6	1060247.3	AVERAGE IRRIGATION DEMAND	0.0
TOTAL SUBLIMATION LOSS	0.0	0.0	MEDIAN IRRIGATION DEMAND	0.0
SNOW PACK AT THE END OF SIMULATION	6.6	1390.2	TOTAL RUNON	10066.3
WATER BALANCE (OVER PERIOD OF RECORD)	0.0	0.0	AVERAGE RUNON	197.4
			MEDIAN RUNON	194.9
			TOTAL SEEPAGE	3313.1
			AVERAGE SEEPAGE	65.0
			MEDIAN SEEPAGE	64.8
			TOTAL EVAPORATION	26361.9
			AVERAGE EVAPORATION	516.9
			MEDIAN EVAPORATION	514.4
			TOTAL SUBLIMATION LOSSES	0.0
			SNOW PACK AT THE END OF SIMULATION	6.6
			WATER BALANCE (OVER PERIOD OF RECORD)	-0.1
				-28.8

ANNUAL SUMMARIES

YEAR	IMPERVIOUS SURFACE				PERVIOUS SURFACE				EVAPORATION	TOTAL RUNOFF	% RUNOFF
	MSC PRECIP	TOTAL RUNOFF	% RUNOFF	MAX RUNOFF	TOTAL IRRIGATION	TOTAL RUNON	SEEPAGE				
(mm)	(mm)	(-)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(-)	
1960	373.0	277.4	74.4	34.1	0.0	176.1	23.0	492.2	8.2	2.2	
1961	392.1	305.6	77.9	35.7	0.0	194.0	64.9	475.7	35.4	9.0	
1962	285.3	187.2	65.6	27.4	0.0	118.8	15.2	432.1	0.0	0.0	
1963	425.0	341.6	80.4	41.1	0.0	216.9	69.7	532.3	5.1	1.2	
1964	392.4	283.1	72.1	38.5	0.0	179.7	67.5	445.3	32.5	8.3	
1965	590.2	491.5	83.3	49.2	0.0	312.0	221.6	604.3	82.4	14.0	
1966	403.7	322.4	79.9	53.3	0.0	204.7	64.8	504.5	37.8	9.4	
1967	256.4	181.2	70.7	20.6	0.0	115.0	33.0	328.3	49.8	19.4	
1968	358.6	253.0	70.6	38.3	0.0	160.6	56.2	459.0	0.0	0.0	

1969	428.1	357.5	83.5	28.9	0.0	227.0	112.2	546.5	11.9	2.8
1970	397.8	300.1	75.4	74.3	0.0	190.5	45.8	463.4	59.2	14.9
1971	392.5	296.8	75.6	30.7	0.0	188.4	62.7	502.5	9.5	2.4
1972	482.4	395.7	82.0	35.9	0.0	251.2	75.1	594.2	37.0	7.7
1973	361.5	282.2	78.1	23.3	0.0	179.1	19.9	522.5	17.8	4.9
1974	347.7	259.5	74.6	27.9	0.0	164.7	69.2	470.7	13.4	3.8
1975	369.2	255.2	69.1	30.3	0.0	162.0	14.0	472.3	0.0	0.0
1976	405.8	307.0	75.7	28.4	0.0	194.9	33.4	575.4	2.6	0.6
1977	420.5	300.7	71.5	25.5	0.0	190.9	50.9	576.9	0.0	0.0
1978	533.2	426.1	79.9	53.7	0.0	270.5	131.4	620.7	50.4	9.5
1979	285.2	205.9	72.2	24.4	0.0	130.7	2.3	413.3	0.0	0.0
1980	446.2	341.1	76.4	36.6	0.0	216.5	87.7	540.4	8.2	1.8
1981	507.3	392.4	77.4	50.2	0.0	249.1	80.3	681.6	26.5	5.2
1982	420.9	320.8	76.2	30.2	0.0	203.7	67.7	562.0	0.0	0.0
1983	294.8	189.1	64.1	30.8	0.0	120.0	13.2	396.5	0.0	0.0
1984	367.2	280.8	76.5	39.2	0.0	178.3	85.0	449.3	0.0	0.0
1985	388.7	301.5	77.6	91.0	0.0	191.4	37.3	451.3	87.6	22.5
1986	462.3	362.1	78.3	59.1	0.0	229.9	111.9	510.6	87.5	18.9
1987	351.8	258.5	73.5	21.4	0.0	164.1	75.8	455.1	0.0	0.0
1988	404.7	313.7	77.5	63.4	0.0	199.1	81.4	481.2	35.6	8.8
1989	387.5	269.4	69.5	20.4	0.0	171.0	0.0	529.0	0.0	0.0
1990	398.2	270.0	67.8	27.8	0.0	171.4	56.1	514.4	0.0	0.0
1991	406.6	308.0	75.8	33.0	0.0	195.5	89.3	534.7	0.0	0.0
1992	494.9	360.9	72.9	43.0	0.0	229.1	76.1	529.5	63.2	12.8
1993	449.5	353.1	78.6	36.6	0.0	224.2	100.0	616.7	18.8	4.2
1994	355.1	258.1	72.7	24.2	0.0	163.8	17.0	482.6	0.0	0.0
1995	414.8	308.3	74.3	33.8	0.0	195.7	62.9	506.4	0.0	0.0
1996	376.4	266.0	70.7	28.8	0.0	168.9	42.4	470.8	30.9	8.2
1997	425.2	353.3	83.1	40.4	0.0	224.3	93.1	540.0	81.3	19.1
1998	537.7	431.0	80.2	52.0	0.0	273.6	149.8	554.8	80.3	14.9
1999	458.6	362.0	78.9	50.8	0.0	229.8	123.8	568.7	23.5	5.1
2000	412.6	312.9	75.8	30.0	0.0	198.6	30.8	572.9	0.0	0.0
2001	318.4	244.8	76.9	25.8	0.0	155.4	36.5	407.3	12.8	4.0
2002	344.5	241.4	70.1	24.4	0.0	153.2	2.1	495.6	0.0	0.0
2003	430.0	343.6	79.9	26.8	0.0	218.1	63.1	576.6	0.0	0.0
2004	386.8	262.2	67.8	44.2	0.0	166.5	18.3	550.3	0.0	0.0
2005	539.6	445.0	82.5	63.2	0.0	282.5	137.8	553.2	140.9	26.1
2006	419.6	320.3	76.3	31.0	0.0	203.3	92.9	493.2	8.2	2.0
2007	511.4	399.8	78.2	70.8	0.0	253.8	87.1	631.1	69.2	13.5
2008	502.6	368.8	73.4	33.4	0.0	234.1	87.4	595.8	13.1	2.6
2009	328.0	232.4	70.9	27.8	0.0	147.5	28.5	436.1	28.2	8.6
2010	454.5	355.8	78.3	32.8	0.0	225.9	45.2	641.9	0.0	0.0

STORAGE/ REUSE TANK	(m ³)	TOTAL AREA
MAXIMUM VOLUME	0.0	TOTAL IMPERVIOUS AREA
MINIMUM VOLUME	0.0	DIRECTLY CONNECTED IMPERVIOUS AREA
TOTAL INFLOW	0.0	IMPERVIOUS TO ABSORBENT LANDSCAPING
TOTAL DEMAND	0.0	IMPERVIOUS TO BIORETENTION
TOTAL OVERFLOW	0.0	PERVIOUS AREA
TOTAL MUN. MAKE-UP WATER	0.0	ABSORBENT LANDSCAPING AREA
TOTAL RECHARGE	0.0	GREEN ROOF AREA
		BIORETENTION AREA

SC1: ABSORBENT AREA	(mm)	(m3)	SC1: GREENROOF	(mm)
TOTAL MSC PRECIPITATION	20897.0	0.0	TOTAL MSC PRECIPITATION	20897.0
TOTAL RUNOFF	2415.9	0.0	TOTAL RUNOFF	726.2
% OF RAINFALL AS RUNOFF	11.6		% OF RAINFALL AS RUNOFF	3.5
AVERAGE RUNOFF	47.4	0.0	AVERAGE RUNOFF	14.2
MEDIAN RUNOFF	40.7	0.0	MEDIAN RUNOFF	8.2
MAXIMUM RUNOFF (ANY TIMESTEP)	72.1	0.0	MAXIMUM RUNOFF (ANY TIMESTEP)	42.4
TOTAL IRRIGATION DEMAND	0.0	0.0	TOTAL IRRIGATION DEMAND	0.0
AVERAGE IRRIGATION DEMAND	0.0	0.0	AVERAGE IRRIGATION DEMAND	0.0
MEDIAN IRRIGATION DEMAND	0.0	0.0	MEDIAN IRRIGATION DEMAND	0.0
TOTAL RUNON	0.0	0.0	TOTAL RUNON	0.0
AVERAGE RUNON	0.0	0.0	AVERAGE RUNON	0.0
MEDIAN RUNON	0.0	0.0	MEDIAN RUNON	0.0
TOTAL SEEPAGE	4454.9	0.0	TOTAL SEEPAGE	20110.1
AVERAGE SEEPAGE	87.4	0.0	AVERAGE SEEPAGE	394.3
MEDIAN SEEPAGE	82.5	0.0	MEDIAN SEEPAGE	390.4
TOTAL EVAPORATION	14043.4	0.0	TOTAL EVAPORATION	0.0
AVERAGE EVAPORATION	275.4	0.0	AVERAGE EVAPORATION	0.0
MEDIAN EVAPORATION	277.5	0.0	MEDIAN EVAPORATION	0.0
TOTAL SUBLIMATION LOSSES	0.0	0.0	TOTAL SUBLIMATION LOSSES	0.0
SNOW PACK AT THE END OF SIMULATION	6.6	0.0	SNOW PACK AT THE END OF SIMULATION	6.6
WATER BALANCE (OVER PERIOD OF RECORD)	-39.5	0.0	WATER BALANCE (OVER PERIOD OF RECORD)	0.0

ABSORBENT LANDSCAPING

GREEN ROOF MEDIA

MAX RUNOFF	TOTAL IRRIGATION	TOTAL RUNON	SEEPAGE	EVAPORATION	TOTAL RUNOFF	% RUNOFF	MAX RUNOFF	TOTAL IRRIGATION	TOTAL RUNON	SEEPAGE	EVAPORATION
(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(-)	(mm)	(mm)	(mm)	(mm)	(mm)
8.2	0.0	0.0	69.8	263.9	20.9	5.6	16.5	0.0	0.0	312.9	0.0
18.7	0.0	0.0	104.5	254.4	41.0	10.5	17.7	0.0	0.0	386.7	0.0
0.0	0.0	0.0	53.2	224.3	14.4	5.1	8.5	0.0	0.0	292.3	0.0
5.1	0.0	0.0	77.7	264.5	70.5	16.6	23.9	0.0	0.0	402.1	0.0
26.2	0.0	0.0	67.5	238.8	62.8	16.0	19.6	0.0	0.0	378.4	0.0
22.4	0.0	0.0	152.5	315.3	140.1	23.7	30.3	0.0	0.0	573.7	0.0
15.7	0.0	0.0	91.8	233.0	79.4	19.7	34.6	0.0	0.0	390.4	0.0
20.2	0.0	0.0	50.4	177.3	40.7	15.9	24.6	0.0	0.0	236.3	0.0
0.0	0.0	0.0	82.4	225.9	41.5	11.6	22.6	0.0	0.0	357.2	0.0

9.6	0.0	0.0	103.7	294.7	51.1	11.9	12.9	0.0	0.0	436.0	0.0
49.0	0.0	0.0	82.1	241.5	55.4	13.9	55.4	0.0	0.0	355.8	0.0
9.1	0.0	0.0	78.9	256.5	57.4	14.6	15.2	0.0	0.0	376.4	0.0
35.1	0.0	0.0	90.2	307.0	77.2	16.0	19.0	0.0	0.0	439.4	0.0
14.8	0.0	0.0	73.6	263.8	40.3	11.2	17.0	0.0	0.0	352.1	0.0
11.7	0.0	0.0	73.4	251.1	43.3	12.4	24.7	0.0	0.0	347.0	0.0
0.0	0.0	0.0	48.3	274.0	16.6	4.5	11.4	0.0	0.0	335.1	0.0
2.6	0.0	0.0	60.3	323.0	30.9	7.6	9.8	0.0	0.0	397.0	0.0
0.0	0.0	0.0	115.6	277.5	31.4	7.5	9.2	0.0	0.0	424.7	0.0
20.6	0.0	0.0	128.8	320.6	89.1	16.7	40.0	0.0	0.0	512.0	0.0
0.0	0.0	0.0	56.5	225.7	8.8	3.1	5.5	0.0	0.0	285.6	0.0
8.2	0.0	0.0	129.7	282.1	31.4	7.0	17.7	0.0	0.0	425.2	0.0
16.3	0.0	0.0	141.8	332.2	52.8	10.4	31.3	0.0	0.0	497.7	0.0
0.0	0.0	0.0	95.5	304.5	17.2	4.1	11.3	0.0	0.0	409.8	0.0
0.0	0.0	0.0	49.5	215.7	16.4	5.5	11.9	0.0	0.0	284.6	0.0
0.0	0.0	0.0	75.1	275.7	25.6	7.0	20.3	0.0	0.0	368.5	0.0
87.6	0.0	0.0	65.3	245.6	77.4	19.9	72.1	0.0	0.0	345.2	0.0
53.0	0.0	0.0	103.6	276.5	88.8	19.2	40.2	0.0	0.0	462.6	0.0
0.0	0.0	0.0	111.8	242.2	3.8	1.1	2.5	0.0	0.0	356.5	0.0
35.5	0.0	0.0	82.5	259.6	63.5	15.7	48.5	0.0	0.0	381.2	0.0
0.0	0.0	0.0	46.6	322.7	4.4	1.1	2.0	0.0	0.0	358.7	0.0
0.0	0.0	0.0	75.7	309.6	8.9	2.2	8.9	0.0	0.0	398.0	0.0
0.0	0.0	0.0	91.1	298.7	34.8	8.6	14.1	0.0	0.0	396.1	0.0
39.5	0.0	0.0	95.3	293.9	78.6	15.9	30.5	0.0	0.0	461.4	0.0
9.9	0.0	0.0	111.9	308.0	60.0	13.4	17.7	0.0	0.0	454.2	0.0
0.0	0.0	0.0	70.6	264.7	12.9	3.6	9.2	0.0	0.0	350.2	0.0
0.0	0.0	0.0	71.4	296.8	36.8	8.9	20.4	0.0	0.0	385.4	0.0
22.3	0.0	0.0	89.4	228.3	34.8	9.2	16.6	0.0	0.0	336.2	0.0
24.1	0.0	0.0	80.9	283.9	101.2	23.8	23.6	0.0	0.0	433.6	0.0
51.5	0.0	0.0	141.5	305.6	74.7	13.9	35.0	0.0	0.0	472.3	0.0
13.5	0.0	0.0	103.1	297.9	76.4	16.7	31.9	0.0	0.0	478.7	0.0
0.0	0.0	0.0	94.4	288.9	19.4	4.7	11.1	0.0	0.0	399.9	0.0
12.8	0.0	0.0	71.5	219.1	22.9	7.2	11.5	0.0	0.0	301.3	0.0
0.0	0.0	0.0	57.2	283.8	8.2	2.4	5.5	0.0	0.0	333.8	0.0
0.0	0.0	0.0	105.3	294.4	28.1	6.5	7.9	0.0	0.0	411.4	0.0
0.0	0.0	0.0	44.7	313.6	31.3	8.1	25.3	0.0	0.0	389.8	0.0
59.9	0.0	0.0	125.3	288.9	134.0	24.8	48.8	0.0	0.0	546.4	0.0
4.7	0.0	0.0	110.2	245.3	48.2	11.5	18.0	0.0	0.0	397.3	0.0
41.4	0.0	0.0	85.0	328.9	107.8	21.1	51.9	0.0	0.0	474.3	0.0
11.5	0.0	0.0	116.3	329.7	26.5	5.3	14.5	0.0	0.0	465.7	0.0
13.2	0.0	0.0	53.0	253.6	32.8	10.0	24.7	0.0	0.0	283.9	0.0
0.0	0.0	0.0	98.2	320.0	43.1	9.5	14.0	0.0	0.0	459.4	0.0

45.8 ha	
21.1 ha	0.5 of total area
15.7 ha	0.7 of total impervious area
21.1 ha	1.0 of total impervious area
21.1 ha	1.0 of total impervious area
24.8 ha	0.5 of total area
0.0 ha	0.0 of total area
0.0 ha	0.0 of total area
0.0 ha	0.0 of total area

(m3)	SC1: BIOSWALE	(mm)	(m3)	SC1: STORAGE / REUSE
0.0	TOTAL MSC PRECIPITATION	20897.0	0.0	MAXIMUM VOLUME OVER RECOR
0.0	TOTAL SURFACE RUNOFF	0.0	0.0	AVERAGE MAX. VOLUME
	% OF RAINFALL AS SURFACE RUNOFF + SUBDRAI	0.0		MEDIAN MAX. VOLUME
0.0	AVERAGE SURFACE RUNOFF	0.0	0.0	MINIMUM VOLUME OVER RECORI
0.0	MEDIAN SURFACE RUNOFF	0.0	0.0	AVERAGE MIN. VOLUME
0.0	MAXIMUM RUNOFF (ANY TIMESTEP)	0.0	0.0	MEDIAN MIN. VOLUME
0.0	TOTAL RUNON	0.0	0.0	TOTAL INFLOW
0.0	AVERAGE RUNON	0.0	0.0	AVERAGE INFLOW
0.0	MEDIAN RUNON	0.0	0.0	MEDIAN INFLOW
0.0	TOTAL PERCOLATION	245.1	0.0	TOTAL DEMAND
0.0	AVERAGE PERCOLATION	4.8	0.0	AVERAGE DEMAND
0.0	MEDIAN PERCOLATION	0.1	0.0	MEDIAN DEMAND
0.0	TOTAL EVAPORATION	20635.5	0.0	TOTAL OVERFLOW
0.0	AVERAGE EVAPORATION	404.6	0.0	AVERAGE OVERFLOW
0.0	MEDIAN EVAPORATION	398.6	0.0	MEDIAN OVERFLOW
0.0	TOTAL SUBDRAIN	0.0	0.0	TOTAL MUN. MAKE-UP WATER
0.0	AVERAGE SUBDRAIN	0.0	0.0	AVERAGE MUN. MAKE-UP WATEF
0.0	MEDIAN SUBDRAIN	0.0	0.0	MEDIAN MUN. MAKE-UP WATER
0.0	TOTAL SUBLIMATION LOSSES	0.0	0.0	TOTAL RECHARGE FROM PONDS
0.0	SNOW PACK AT THE END OF SIMULATION	6.6	0.0	AVERAGE RECHARGE FROM PON
0.0	WATER BALANCE (OVER PERIOD OF RECORD)	-0.1	0.0	MEDIAN RECHARGE FROM POND WATER BALANCE (OVER PERIOD

BIOSWALE / BIORETENTION MEDIA

STORAGE / REUSE SYSTEM

TOTAL RUNOFF % RUNOFF MAX RUNOFF TOTAL RUNON PERCOLATION EVAPORATION SUBDRAIN TOTAL RUNOFF % RUNOFF MAX RUNOFF TOTAL INFLOW MAX VOLUME

(mm)	(-)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(-)	(mm)	(m3)	(m3)
0.0	0.0	0.0	0.0	0.0	356.2	0.0	0.0	0.0	0.0	0.0	0.0
17.1	4.4	7.4	0.0	1.6	390.9	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	300.3	0.0	0.0	0.0	0.0	0.0	0.0
4.9	1.1	3.9	0.0	0.1	405.3	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.4	370.8	0.0	0.0	0.0	0.0	0.0	0.0
21.0	3.6	6.3	0.0	26.5	572.4	0.0	0.0	0.0	0.0	0.0	0.0
13.3	3.3	7.2	0.0	1.7	398.6	0.0	0.0	0.0	0.0	0.0	0.0
32.5	12.7	11.0	0.0	13.2	271.0	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.1	350.4	0.0	0.0	0.0	0.0	0.0	0.0

8.2	1.9	4.1	0.0	1.0	448.9	0.0	0.0	0.0	0.0	0.0	0.0
28.0	7.1	27.9	0.0	22.8	352.8	0.0	0.0	0.0	0.0	0.0	0.0
7.3	1.8	5.1	0.0	0.2	387.8	0.0	0.0	0.0	0.0	0.0	0.0
31.5	6.5	23.8	0.0	0.1	471.9	0.0	0.0	0.0	0.0	0.0	0.0
24.5	6.8	13.2	0.0	0.0	367.9	0.0	0.0	0.0	0.0	0.0	0.0
22.7	6.5	9.2	0.0	0.0	378.1	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	334.2	0.0	0.0	0.0	0.0	0.0	0.0
17.5	4.3	8.6	0.0	0.0	417.9	0.0	0.0	0.0	0.0	0.0	0.0
10.1	2.4	3.9	0.0	0.0	426.1	0.0	0.0	0.0	0.0	0.0	0.0
19.3	3.6	6.9	0.0	1.9	535.9	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	287.5	0.0	0.0	0.0	0.0	0.0	0.0
5.3	1.2	3.6	0.0	3.2	427.3	0.0	0.0	0.0	0.0	0.0	0.0
32.8	6.5	15.4	0.0	2.7	530.9	0.0	0.0	0.0	0.0	0.0	0.0
14.8	3.5	4.2	0.0	0.0	416.8	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	282.1	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	1.2	354.4	0.0	0.0	0.0	0.0	0.0	0.0
42.4	10.9	42.4	0.0	25.2	362.2	0.0	0.0	0.0	0.0	0.0	0.0
7.8	1.7	7.8	0.0	35.7	436.8	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	370.8	0.0	0.0	0.0	0.0	0.0	0.0
17.9	4.4	17.9	0.0	5.4	399.2	0.0	0.0	0.0	0.0	0.0	0.0
3.9	1.0	2.5	0.0	0.0	370.2	0.0	0.0	0.0	0.0	0.0	0.0
5.3	1.3	1.5	0.0	0.1	394.6	0.0	0.0	0.0	0.0	0.0	0.0
22.8	5.6	10.4	0.0	0.0	425.4	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	15.3	437.7	0.0	0.0	0.0	0.0	0.0	0.0
36.8	8.2	12.7	0.0	0.1	493.3	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	348.4	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	385.0	0.0	0.0	0.0	0.0	0.0	0.0
37.3	9.9	12.3	0.0	0.0	371.2	0.0	0.0	0.0	0.0	0.0	0.0
35.7	8.4	9.5	0.0	1.2	466.0	0.0	0.0	0.0	0.0	0.0	0.0
37.4	7.0	31.3	0.0	16.8	504.5	0.0	0.0	0.0	0.0	0.0	0.0
2.1	0.5	2.1	0.0	1.6	476.1	0.0	0.0	0.0	0.0	0.0	0.0
14.5	3.5	8.4	0.0	0.0	402.2	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.6	311.8	0.0	0.0	0.0	0.0	0.0	0.0
10.9	3.2	7.1	0.0	0.0	348.0	0.0	0.0	0.0	0.0	0.0	0.0
16.4	3.8	8.3	0.0	0.0	427.3	0.0	0.0	0.0	0.0	0.0	0.0
5.5	1.4	4.0	0.0	0.0	389.8	0.0	0.0	0.0	0.0	0.0	0.0
2.4	0.4	2.4	0.0	48.3	499.2	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	1.2	402.6	0.0	0.0	0.0	0.0	0.0	0.0
54.3	10.6	21.7	0.0	16.4	505.3	0.0	0.0	0.0	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.4	470.0	0.0	0.0	0.0	0.0	0.0	0.0
60.5	18.5	24.2	0.0	0.0	339.8	0.0	0.0	0.0	0.0	0.0	0.0
1.4	0.3	0.6	0.0	0.0	461.5	0.0	0.0	0.0	0.0	0.0	0.0

(m3)

D	0.0
	0.0
	0.0
J	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
	0.0
R	0.0
	0.0
	0.0
IDS	0.0
S	0.0
OF RECORD)	0.0

SUB CATCHMEN EXTERNAL RUN-ON

MIN VOLUME (m3)	TOTAL DEMAND (m3)	TOTAL OVERFLOW (m3)	TOTAL MUNICIPAL MAKE-UP WATER (m3)	RECHARGE VOLUME (m3)	DISCHARGE (m3)	TOTAL INFLOW VOLUME (m3)
0.0	0.0	0.0	0.0	0.0	16824.8	0.0
0.0	0.0	0.0	0.0	0.0	25068.3	0.0
0.0	0.0	0.0	0.0	0.0	9976.0	0.0
0.0	0.0	0.0	0.0	0.0	19474.0	0.0
0.0	0.0	0.0	0.0	0.0	23145.5	0.0
0.0	0.0	0.0	0.0	0.0	46618.2	0.0
0.0	0.0	0.0	0.0	0.0	26550.5	0.0
0.0	0.0	0.0	0.0	0.0	22007.0	0.0
0.0	0.0	0.0	0.0	0.0	13482.5	0.0

0.0	0.0	0.0	0.0	0.0	21988.6	0.0
0.0	0.0	0.0	0.0	0.0	30674.4	0.0
0.0	0.0	0.0	0.0	0.0	18178.1	0.0
0.0	0.0	0.0	0.0	0.0	30260.5	0.0
0.0	0.0	0.0	0.0	0.0	19441.8	0.0
0.0	0.0	0.0	0.0	0.0	17145.7	0.0
0.0	0.0	0.0	0.0	0.0	13599.7	0.0
0.0	0.0	0.0	0.0	0.0	16992.3	0.0
0.0	0.0	0.0	0.0	0.0	16024.4	0.0
0.0	0.0	0.0	0.0	0.0	35201.2	0.0
0.0	0.0	0.0	0.0	0.0	10972.5	0.0
0.0	0.0	0.0	0.0	0.0	20208.3	0.0
0.0	0.0	0.0	0.0	0.0	27483.9	0.0
0.0	0.0	0.0	0.0	0.0	17095.6	0.0
0.0	0.0	0.0	0.0	0.0	10077.2	0.0
0.0	0.0	0.0	0.0	0.0	14963.9	0.0
0.0	0.0	0.0	0.0	0.0	37783.2	0.0
0.0	0.0	0.0	0.0	0.0	40990.3	0.0
0.0	0.0	0.0	0.0	0.0	13775.6	0.0
0.0	0.0	0.0	0.0	0.0	25544.2	0.0
0.0	0.0	0.0	0.0	0.0	14356.4	0.0
0.0	0.0	0.0	0.0	0.0	14388.4	0.0
0.0	0.0	0.0	0.0	0.0	16413.4	0.0
0.0	0.0	0.0	0.0	0.0	34894.4	0.0
0.0	0.0	0.0	0.0	0.0	23485.2	0.0
0.0	0.0	0.0	0.0	0.0	13754.3	0.0
0.0	0.0	0.0	0.0	0.0	16429.4	0.0
0.0	0.0	0.0	0.0	0.0	21828.2	0.0
0.0	0.0	0.0	0.0	0.0	38966.3	0.0
0.0	0.0	0.0	0.0	0.0	42877.0	0.0
0.0	0.0	0.0	0.0	0.0	25123.6	0.0
0.0	0.0	0.0	0.0	0.0	16674.6	0.0
0.0	0.0	0.0	0.0	0.0	16218.6	0.0
0.0	0.0	0.0	0.0	0.0	12864.3	0.0
0.0	0.0	0.0	0.0	0.0	18310.6	0.0
0.0	0.0	0.0	0.0	0.0	13972.7	0.0
0.0	0.0	0.0	0.0	0.0	58628.6	0.0
0.0	0.0	0.0	0.0	0.0	19099.6	0.0
0.0	0.0	0.0	0.0	0.0	38463.6	0.0
0.0	0.0	0.0	0.0	0.0	22894.1	0.0
0.0	0.0	0.0	0.0	0.0	19377.4	0.0
0.0	0.0	0.0	0.0	0.0	18960.7	0.0

POND 1 DISCHARGES TO	POND #1 OUTFALL	CATCHMENT AREA SIZE									
		45.8 ha - DIRECT			45.8 ha - TOTAL						
		MAX	MIN	Avg	Median		MAX	Total	Avg	Median	
VOLUME (m³)	34545.6	7395.9	8669.4	8624.9	1300.6						
LEVEL (m)	1303.5	1300.3	1300.5								
UNIT AREA RESULTS BASED ON TOTAL CATCHMENT SIZE											
INFLOW (m³)	59721.0	1171923.9	22978.9	19533.6	(mm)	130.3	2556.1	50.1	42.6		
DIRECT PRECIPITATION (m³)	3373.2	107016.3	2098.4	2050.4	(mm)	7.4	233.4	4.6	4.5		
EVAPORATION LOSS (m³)	4043.0	188286.3	3691.9	3683.9	(mm)	8.8	410.7	8.1	8.0		
SEEPAGE LOSS (m³)	0.0	0.0	0.0	0.0	(mm)	0.0	0.0	0.0	0.0		
DISCHARGE (m³)	59234.0	1090198.8	21376.4	17533.5	(mm)	129.2	2377.8	46.6	38.2		
OVERFLOW (m³)	0.0	0.0	0.0	0.0	(mm)	0.0	0.0	0.0	0.0		
MAKE-UP WATER (m³)	0.0	0.0	0.0	0.0	(mm)	0.0	0.0	0.0	0.0		
DEMAND (m³)	0.0	0.0	0.0	0.0	(mm)	0.0	0.0	0.0	0.0		
WATER BALANCE (m³)		0.0									

ANNUAL SUMMARIES

YEAR	POND #1											
	VOLUME MAX (m³)	VOLUME MIN (m³)	LEVEL MAX (m)	LEVEL MIN (m)	Inflow (m³)	Direct Precipitation (m³)	Evaporation (m³)	Seepage (m³)	Discharge (m³)	Overflow (m³)	Make-Up Water (m³)	Demand (m³)
1960	11354.5	8174.9	1301.1	1300.5	16938.1	1827.8	3678.0	0.0	14633.6	0.0	0.0	0.0
1961	16319.3	7969.3	1301.9	1300.5	25213.7	1999.9	3737.6	0.0	23475.9	0.0	0.0	0.0
1962	9511.2	8317.2	1300.8	1300.5	9979.8	1399.8	3445.7	0.0	8020.4	0.0	0.0	0.0
1963	10700.8	8067.7	1301.0	1300.5	19580.4	2119.4	3766.0	0.0	17846.0	0.0	0.0	0.0
1964	15790.1	7804.0	1301.8	1300.4	23394.9	1983.1	3499.8	0.0	21880.7	0.0	0.0	0.0
1965	13606.1	8256.1	1301.5	1300.5	47331.6	2996.8	3191.1	0.0	47134.0	0.0	0.0	0.0
1966	14575.1	8162.0	1301.7	1300.5	27057.8	2062.6	3488.2	0.0	25634.6	0.0	0.0	0.0
1967	13563.1	7395.9	1301.5	1300.3	22571.0	1245.8	3558.6	0.0	20571.2	0.0	0.0	0.0
1968	10112.5	8231.3	1300.9	1300.5	13577.9	1774.4	3321.1	0.0	11718.8	0.0	0.0	0.0
1969	11686.8	8279.6	1301.2	1300.5	22134.4	2146.2	3654.0	0.0	20645.6	0.0	0.0	0.0
1970	23756.0	8130.0	1302.8	1300.5	31202.1	2389.8	4008.9	0.0	29565.4	0.0	0.0	0.0
1971	10861.8	8161.3	1301.0	1300.5	18418.9	1936.1	4043.0	0.0	16311.3	0.0	0.0	0.0
1972	17390.2	8377.1	1302.1	1300.5	30676.6	2395.2	3578.2	0.0	29481.9	0.0	0.0	0.0
1973	11897.9	8360.6	1301.2	1300.5	19607.7	1780.1	3928.8	0.0	17470.4	0.0	0.0	0.0
1974	11918.9	8090.1	1301.2	1300.5	17449.4	1717.2	3711.5	0.0	15456.1	0.0	0.0	0.0
1975	9584.6	8331.2	1300.8	1300.5	13599.7	1809.8	3659.8	0.0	11725.8	0.0	0.0	0.0
1976	10173.9	8331.2	1300.9	1300.5	17015.7	2005.0	3958.5	0.0	15080.3	0.0	0.0	0.0
1977	9521.1	8097.3	1300.8	1300.5	16105.1	2072.1	3852.3	0.0	14327.3	0.0	0.0	0.0
1978	15519.0	8476.3	1301.8	1300.6	35795.1	2716.3	3504.5	0.0	35010.2	0.0	0.0	0.0
1979	9483.8	8199.7	1300.8	1300.5	10972.5	1395.4	3693.9	0.0	8677.2	0.0	0.0	0.0
1980	11150.7	8014.0	1301.1	1300.5	20208.3	2212.2	3790.3	0.0	18620.9	0.0	0.0	0.0
1981	15173.3	8290.6	1301.7	1300.5	27780.6	2570.9	3802.9	0.0	26557.0	0.0	0.0	0.0
1982	9614.5	8199.1	1300.8	1300.5	17095.6	2076.5	3643.4	0.0	15527.1	0.0	0.0	0.0
1983	9449.1	8171.2	1300.8	1300.5	10077.2	1447.4	3656.8	0.0	7867.1	0.0	0.0	0.0
1984	9728.8	8102.8	1300.8	1300.5	14989.2	1812.6	3639.9	0.0	13161.8	0.0	0.0	0.0
1985	34545.6	8146.4	1303.5	1300.5	38602.6	2492.7	3794.0	0.0	37304.2	0.0	0.0	0.0
1986	25108.7	8154.2	1302.9	1300.5	41746.7	2746.8	3683.3	0.0	40824.7	0.0	0.0	0.0
1987	9498.2	7940.4	1300.8	1300.4	13775.6	1738.2	3943.5	0.0	11609.3	0.0	0.0	0.0
1988	19656.1	7832.8	1302.4	1300.4	25983.8	2148.0	3940.9	0.0	24140.1	0.0	0.0	0.0
1989	9293.6	8390.1	1300.7	1300.5	14356.4	1898.8	3652.2	0.0	12586.9	0.0	0.0	0.0
1990	9626.8	8390.0	1300.8	1300.5	14388.4	1964.0	3602.3	0.0	12762.4	0.0	0.0	0.0
1991	9698.3	8135.0	1300.8	1300.5	16413.4	2010.4	3741.5	0.0	14711.1	0.0	0.0	0.0
1992	24279.0	8328.7	1302.8	1300.5	35429.2	2663.1	3523.0	0.0	34538.8	0.0	0.0	0.0
1993	11006.8	8371.3	1301.1	1300.5	23854.2	2239.1	3395.4	0.0	22701.6	0.0	0.0	0.0
1994	9633.8	8259.6	1300.8	1300.5	13754.3	1750.9	3782.5	0.0	11719.7	0.0	0.0	0.0
1995	9814.5	8469.7	1300.8	1300.6	16501.0	2050.4	3503.8	0.0	15048.1	0.0	0.0	0.0
1996	13783.9	8093.5	1301.5	1300.5	22109.4	1844.7	3448.2	0.0	20501.5	0.0	0.0	0.0
1997	15997.2	8220.8	1301.9	1300.5	39576.9	2173.3	3761.2	0.0	37995.3	0.0	0.0	0.0
1998	23058.4	8177.5	1302.7	1300.5	43489.0	2958.5	3884.6	0.0	42554.4	0.0	0.0	0.0
1999	14453.6	8344.0	1301.6	1300.5	25387.9	2336.6	3513.2	0.0	24245.7	0.0	0.0	0.0
2000	9703.0	8379.4	1300.8	1300.5	16674.6	2039.2	3873.4	0.0	14811.1	0.0	0.0	0.0
2001	12535.5	8021.7	1301.3	1300.5	16301.3	1584.3	3994.8	0.0	13893.8	0.0	0.0	0.0
2002	9424.5	8153.1	1300.8	1300.5	12864.3	1687.5	3671.2	0.0	10875.5	0.0	0.0	0.0
2003	9657.4	8322.5	1300.8	1300.5	18310.6	2124.7	3684.4	0.0	16760.5	0.0	0.0	0.0
2004	10194.5	8430.3	1300.9	1300.6	14067.5	1907.3	3688.7	0.0	12276.6	0.0	0.0	0.0
2005	30400.2	8274.3	1303.3	1300.5	59721.0	3373.2	3867.6	0.0	59234.0	0.0	0.0	0.0
2006	10764.5	8134.9	1301.0	1300.5	19335.2	2096.1	3674.6	0.0	17697.7	0.0	0.0	0.0
2007	25904.2	8310.8	1303.0	1300.5	39088.9	2933.1	3795.4	0.0	38281.2	0.0	0.0	0.0
2008	11976.2	8417.9	1301.2	1300.6	22920.8	2502.4	3702.1	0.0	21718.0	0.0	0.0	0.0
2009	11840.8	8124.2	1301.2	1300.5	19533.6	1609.3	3668.0	0.0	17472.7	0.0	0.0	0.0
2010	9761.0	8331.5	1300.8	1300.5	18964.0	2250.3	3683.9	0.0	17533.5	0.0	0.0	0.0

POND #2		CATCHMENT AREA SIZE				
DISCHARGES TO		0.0 ha - DIRECT 0.0 ha - TOTAL				
		MAX	MIN	AVG	MEDIAN	
VOLUME (m ³)		0.0	0.0	0.0	0.0	
LEVEL (m)		0.0	0.0	0.0	0.0	UNIT AREA RESULTS BASED ON TOTAL CATCHMENT SIZE
		MAX	TOTAL	Avg	Median	
INFLOW (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
DIRECT PRECIPITATION (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
EVAPORATION LOSS (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
SEEPAGE LOSS (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
DISCHARGE (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
OVERFLOW (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
MAKE-UP WATER (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
Demand (m ³)		0.0	0.0	0.0	0.0	(mm) N/A
WATER BALANCE (m ³)		0.0				

POND #1 -> POND #2		POND #2 -> POND #1		OUTFALL	
TOT VOLUME (m3)	MAX VOLUME (m3)	TOT VOLUME (m3)	MAX VOLUME (m3)	TOT VOLUME (m3)	MAX VOLUME (m3)
0.0	0.0	0.0	0.0	14633.6	1284.0
0.0	0.0	0.0	0.0	23475.9	3052.8
0.0	0.0	0.0	0.0	8020.4	552.1
0.0	0.0	0.0	0.0	17846.0	1161.3
0.0	0.0	0.0	0.0	21880.7	2774.6
0.0	0.0	0.0	0.0	47134.0	2227.0
0.0	0.0	0.0	0.0	25634.6	2902.8
0.0	0.0	0.0	0.0	20571.2	2219.3
0.0	0.0	0.0	0.0	11718.8	942.7
0.0	0.0	0.0	0.0	20645.6	1585.8
0.0	0.0	0.0	0.0	29565.4	4030.6
0.0	0.0	0.0	0.0	16311.3	1110.1
0.0	0.0	0.0	0.0	29481.9	3111.1
0.0	0.0	0.0	0.0	17470.4	1512.1
0.0	0.0	0.0	0.0	15456.1	1520.9
0.0	0.0	0.0	0.0	11725.8	602.2
0.0	0.0	0.0	0.0	15080.3	1065.1
0.0	0.0	0.0	0.0	14327.3	559.1
0.0	0.0	0.0	0.0	35010.2	3096.5
0.0	0.0	0.0	0.0	8677.2	536.0
0.0	0.0	0.0	0.0	18620.9	1468.0
0.0	0.0	0.0	0.0	26557.0	2777.2
0.0	0.0	0.0	0.0	15527.1	620.7
0.0	0.0	0.0	0.0	7867.1	622.2
0.0	0.0	0.0	0.0	13161.8	761.8
0.0	0.0	0.0	0.0	37304.2	4742.8
0.0	0.0	0.0	0.0	40824.7	4265.4
0.0	0.0	0.0	0.0	11609.3	607.4
0.0	0.0	0.0	0.0	24140.1	3504.1
0.0	0.0	0.0	0.0	12586.9	419.0
0.0	0.0	0.0	0.0	12762.4	634.0
0.0	0.0	0.0	0.0	14711.1	685.8
0.0	0.0	0.0	0.0	34538.8	4045.7
0.0	0.0	0.0	0.0	22701.6	1133.9
0.0	0.0	0.0	0.0	11719.7	670.1
0.0	0.0	0.0	0.0	15048.1	744.5
0.0	0.0	0.0	0.0	20501.5	2301.4
0.0	0.0	0.0	0.0	37995.3	2817.5
0.0	0.0	0.0	0.0	42554.4	4089.8
0.0	0.0	0.0	0.0	24245.7	2705.8
0.0	0.0	0.0	0.0	14811.1	671.9
0.0	0.0	0.0	0.0	13893.8	1770.5
0.0	0.0	0.0	0.0	10875.5	498.7
0.0	0.0	0.0	0.0	16760.5	757.5
0.0	0.0	0.0	0.0	12276.6	975.7
0.0	0.0	0.0	0.0	59234.0	4541.0
0.0	0.0	0.0	0.0	17697.7	1309.1
0.0	0.0	0.0	0.0	38281.2	4213.2
0.0	0.0	0.0	0.0	21718.0	1719.6
0.0	0.0	0.0	0.0	17472.7	1486.3
0.0	0.0	0.0	0.0	17533.5	707.9